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NATIONAL DAM INSPECTION PROGRAM. PYMATUNING RESERVOIR (NDI PA-1--ETC(U)  
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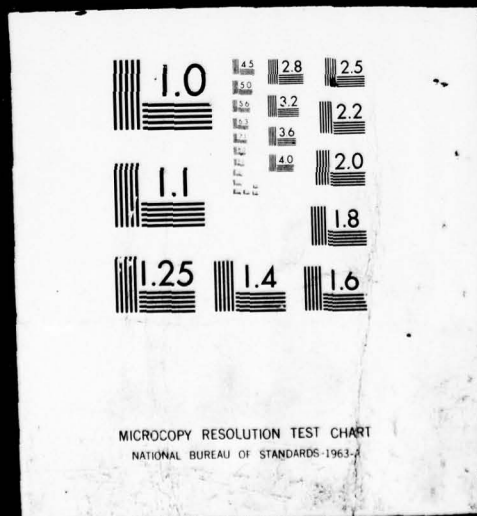
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LEVEL II

OHIO RIVER BASIN  
SHENANGO RIVER, CRAWFORD COUNTY  
PENNSYLVANIA

NDI No. Pa. - 176  
PYMATUNING RESERVOIR

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PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION PROGRAM,

Pymatuning Reservoir (NDI PA-176),  
Ohio River Basin, Shenango River,  
Crawford County, Pennsylvania. Phase I  
Inspection Report.



15 DACW31-78-C-4452

PREPARED FOR

DEPARTMENT OF THE ARMY  
Baltimore District, Corps of Engineers  
Baltimore, Maryland 21203

PREPARED BY

GAI CONSULTANTS, INC.  
570 BEATTY ROAD  
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# LEVEL II

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PHASE I REPORT  
NATIONAL DAM INSPECTION PROGRAM

Pymatuning Reservoir

Pennsylvania

Crawford County

Shenango River

28 July 1978

Inspection Team - GAI Consultants, Inc.

570 Beatty Road

Monroeville, Pennsylvania 15146

Contract No. DACW31-78-C-0052

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Based on a visual inspection, as well as available engineering data, the dam is considered to be in fair condition. A brief hydraulic and hydrologic analysis indicates that the project is capable of passing and/or storing the runoff resulting from a storm of PMF magnitude without overtopping the dam. Thus, the spillway capacity is adequate.

The gate controls for the primary outlet system (consisting of two 6- by 8-foot concrete culverts) are deteriorated and in need of repair and replacement. Also, the deteriorated concrete in the outlet conduits should be repaired.

A swampy area is located along the toe of the embankment near the left abutment. This condition may be the result of poor drainage characteristics downstream of the dam since the structure is reportedly provided with a gravel toe and a blind drain with no apparent outlet. No seepage was observed issuing from the area above the toe at the time of inspection. It is recommended that the swamp be drained and that flow from the area be gauged and recorded. The measurements should then be transmitted to the appropriate agencies within the PennDER for an engineering review of the data. Additional investigations and/or remedial work may be necessary if the weir readings are deemed excessive or anomalous.

It is also recommended that the burrowing animals on the downstream face be exterminated and their burrows filled.

A warning system should be implemented to provide for the safe evacuation of downstream residents in the event that emergency conditions develop and the dam should be inspected on an annual basis to check for hazardous conditions which might develop.

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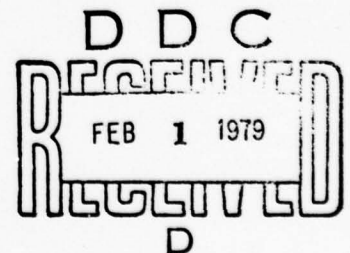
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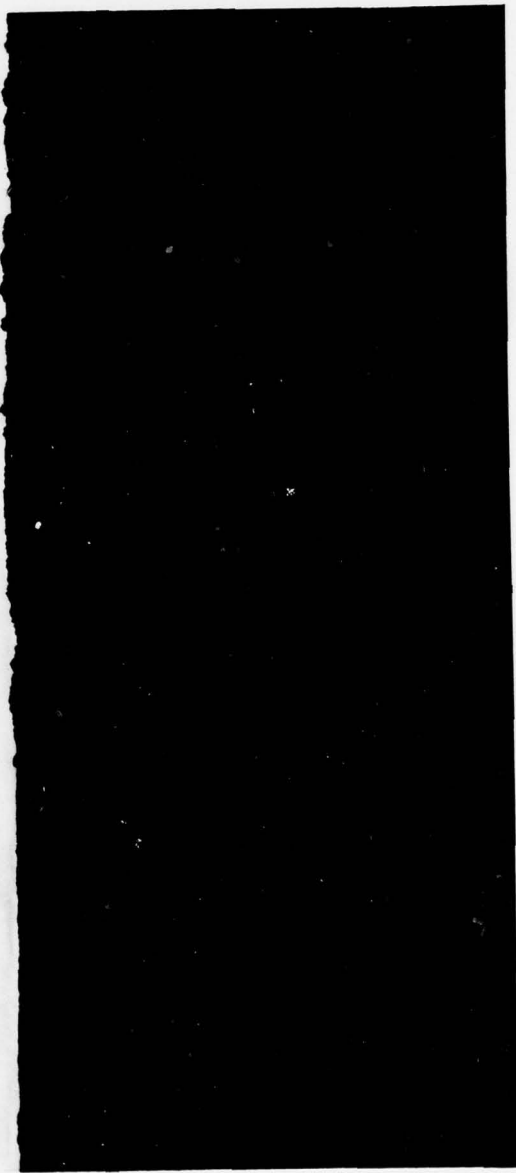
C. K. Withers  
C. K. WITHERS  
Colonel, Corps of Engineers  
District Engineer



Date 13 Sept 78

Date 22 Sep 78





Overview Photograph of Pymatuning Dam

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PHASE I INSPECTION REPORT  
NATIONAL DAM INSPECTION PROGRAM  
PYMATUNING RESERVOIR  
NDI# PA-176, PENNDER# 20-7

SECTION 1  
GENERAL INFORMATION

1.0 Authority.

The Dam Inspection Act, Public Law 92-367, authorized the Secretary of the Army, through the Corps of Engineers, to initiate a program of inspection of dams throughout the United States.

1.1 Purpose.

The purpose is to determine if the dam constitutes a hazard to human life or property.

1.2 Description of Project.

a. Dam and Appurtenances. The dam, located at Pymatuning Reservoir, is an earth embankment approximately 2,400 feet in length with a maximum height of 50 feet. The facility is equipped with both a primary outlet works and an emergency spillway. The primary outlet works consists of two 6 feet by 8 feet concrete box conduits with intakes located at the gate house. Flow is regulated by means of four manually controlled sluice gates operated from within the gate house.

The emergency spillway has a masonry-lined approach channel equipped with a concrete, ogee-shaped weir crest. Flashboards have been installed atop the crest of the weir. Immediately downstream of the weir, the spillway channel is spanned by a pier-supported concrete road bridge.

b. Location. The dam is located along the Shenango River Channel in the southern portion of Crawford County, Pennsylvania, approximately 500 feet northeast of U. S. Route 322. The community of Jamestown, Pennsylvania, is located approximately 1.5 miles southeast of the embankment. The dam, reservoir, and watershed are contained within the Greenville West, Linesville, Hartstown, Conneaut Lake, Harmonsburg, and Beaver Center U.S.G.S. 7.5 minute Pennsylvania quadrangles. In addition, a portion of the reservoir and watershed are contained within the Leon, Pierpont, Andover, and Kinsman U.S.G.S. 7.5 minute Ohio quadrangles. The coordinates of the dam are N41 $\frac{1}{2}$  29.9' and W80 $\frac{1}{2}$  27.7'.



c. Size Classification. Large (50 feet high, 188,000 acre-feet storage capacity at normal pool).

d. Hazard Classification. High. (See section 3.1.c.4.)

e. Ownership. Pennsylvania Department of Environmental Resources.

f. Purpose of Dam. The Pymatuning Act specifically provides that the primary purpose of the Pymatuning Reservoir shall be for the conservation of waters entering the Pymatuning Swamp and for regulating the flow of water in the Shenango and Beaver Rivers. The use of the reservoir for recreation and flood control are secondary purposes made possible under the act.

g. Historical Data\*. Under instructions contained in the Act of the General Assembly approved the 14th day of June, 1911, the Water Supply Commission of Pennsylvania made preliminary surveys and studies to examine the feasibility of converting Pymatuning Swamp into a storage reservoir, and submitted a report to the legislature in 1913, stating that, "As a result of this study, the Commission is assured of the desirability of its construction, and the feasibility thereof, the resulting benefits accruing to the State at large, as well as to the various communities along the stream."

In the years following, there were many legislative acts to provide funds and establish authority for continued work on the project. Through these acts, and with the help of private interests, appropriations were made so that all necessary lands could be purchased, including 4,830 acres in the state of Ohio. Legislation was enacted in that state authorizing the state of Pennsylvania to flood the portion of the proposed reservoir now located in Ohio.

The contract for construction of the dam was awarded on September 8, 1931, and the dam was completed in 1933. Other parts of the project, including the Andover-Esperville Causeway and the constant level weir, and railway fills near Linesville, were completed in 1934. Permanent regulating gates to control releases from the reservoir, were installed in the fall of 1933.

### 1.3 Pertinent Data.

a. Drainage Area. 158 square miles.

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\*The above is an abridged version of a section in the "Operations and Maintenance Manual for Pymatuning Reservoir."

b. Discharge at Dam Site. Discharge records are available at this facility; however, the maximum high water level was not readily ascertainable. Conversations with Pymatuning State Park personnel present the day of inspection, indicated that the emergency spillway has never discharged.

Outlet Works Conduit at Operating Pool Elevation - Discharge curve not available.

Ungated Spillway Capacity at Maximum Pool Elevation  $\approx$  17,150 cfs.

c. Elevation (feet above mean sea level).

Top of Dam - 1,020.

Maximum Pool Design Surcharge - Not known.

Maximum Pool of Record - Not known.

Normal Pool - 1,008 (spillway crest) (1,010 top of flashboards).

Upstream Portal Invert Outlet Conduit  $\approx$  973.5.

Downstream Portal Invert Outlet Conduit - 973.

Streambed at Centerline of Dam  $\approx$  970.

Maximum Tailwater - Not known.

d. Reservoir Length (miles).

Maximum Pool  $\approx$  19 (elevation 1020, top of dam).

Normal Pool  $\approx$  16 (elevation 1008, ogee spillway crest).

e. Reservoir Storage (acre-feet).

Spillway Crest  $\approx$  188,000 (elevation 1,008).

Top of Flashboards  $\approx$  217,000 (elevation 1,010).

Top of Dam  $\approx$  445,000 (elevation 1,020).

Design Surcharge - Not known.

f. Reservoir Surface (acres).

Spillway Crest - 14,528 (elevation 1008).

Top of Dam = 21,000.

Maximum Design Pool - Not known.

g. Dam.

Type - Earth.

Length of Embankment - 1,500 feet (elevation 1,008).

Length, Including Road and Spillway - 2,400 feet.

Height - 50 feet.

Top Width - 32 feet.

Side Slopes - Upper Upstream	2.5H:1V
Middle Upstream	3H:1V
Lower Upstream	2H:1V
Upper Downstream	2.5H:1V
Middle Downstream	3H:1V
Lower Downstream	3H:1V

Zoning - Available drawings indicate the embankment is composed of either rolled or semi-hydraulic fill. Hand placed riprap covers the upstream slope between elevations 995 and 1016. A coarse gravel drain is located at the toe of the dam (see Figure 3) in the valley where the embankment height is greatest.

Impervious Core - (see Figure 3).

Cutoff - Available drawings indicate a cutoff trench was excavated along the centerline of the dam and backfilled with impervious material. Beyond the trench, steel sheetpiling was apparently driven to rock beneath that portion of the dam to the west of the original Shenango River Channel. To the east of the original Shenango River channel, sheetpiling was driven to depth of approximately 60 feet (not to the top of rock), as shown in Figure 3. Upstream of the sheetpiling in this area, a rolled fill impervious blanket was placed for a distance of approximately 800 feet in an apparent attempt to reduce the seepage losses beneath the dam in this area. (see Figure 2)

Grout Curtain - None indicated.

h. Outlet Conduit.

Type - Two 6 feet by 8 feet rectangular box conduits constructed of reinforced concrete.

Length  $\approx$  250 feet.

Closure - Four sluice gates located at the gate house.

Access - Intake is submerged. Gate controls are located within the gate house.

Regulating Facilities - Discharge is controlled via four gate valves which operate 2-1/2 by 4 feet sluice gates.

i. Spillway.

Type - A concrete ogee-shaped weir at the spillway entrance discharges into a rubble masonry-lined channel located at the right abutment.

Spillway Crest - 1008 feet (1010 top of flashboards).

Weir Length - 114 feet.

Channel Length  $\approx$  480 feet.

Upstream Channel - Curved channel lined with hand placed riprap (see Photograph 9).

Downstream Channel - Shenango River Channel.

j. Regulating Facilities - The drawings indicate and a field check substantiated the presence of four sluice gate controls on the first floor of the gate house. The gates control flow through two 6- by 8-foot concrete culverts founded on rock which pass beneath the dam and discharge to the right of center of the dam.



## SECTION 2 ENGINEERING DATA

### 2.1 Design.

#### a. Design Data Availability and Sources.

1. Hydrology and Hydraulics. No engineering data were available concerning hydraulics and hydrology.
2. Embankment. Design drawings (pre-construction) are available from the PennDER files. The contract proposal and specifications were also provided by the Bureau of State Parks in Harrisburg, Pennsylvania.
3. Appurtenant Structures. Same as 2 (above).

b. Design Features. Pre-construction drawings and other files indicate that the dam is a zoned earth embankment. Portions of the embankment and an upstream impervious blanket were constructed of rolled earth as shown on Figure 3. It is not known, however, if the main portion of the embankment was constructed of rolled fill or semi-hydraulic fill. In any case, the specifications state that "the outer portion of the dam shall be composed of coarse material which shall be permanently stable under a condition of complete saturation. The center portion of the dam shall be composed of material containing sufficient clay or other finely divided particles to be highly impervious to water".

Interlocking sheetpiling was reportedly driven to rock, within a cutoff trench, west of old Shenango River channel. To the east, the sheetpiling was driven to an average depth of 60 feet and an impervious blanket was provided upstream of the dam in an attempt to reduce seepage losses in this area (see Figure 2).

The downstream slope of Pymatuning Dam is grass covered. Slopes are 2.5H to 1V between elevations 1002 and 1020 and are 3H to 1V below elevation 1002. Two ten-foot wide berms are located at elevations 1002 and 984 on the downstream dam face. The upstream slope is riprap covered between elevations 995 and 1016. Details of the upstream slope are shown on Figure 3.

The crest of the embankment is protected by masonry guardrails and an asphalt roadway which serves as an access road to park facilities.



## 2. Appurtenant Structures.

a) Spillway. The Pymatuning Reservoir spillway is a concrete and masonry structure with an ogee-shaped crest located at the right abutment (see Figures 3 and 4 and Photographs 8 and 9).

Available pre-construction drawings indicate that the structure is founded on rock and has a cutoff wall on each end.

b) Outlet Works. Controlled release from the reservoir is discharged through two 6-foot by 8-foot rectangular concrete conduits. Flow is controlled within the gate house located to the right of dam center (see Photographs 2 and 4) via four sluice gates measuring 2-1/2 feet wide by 4 feet high.

### 2.2 Construction Records.

No construction records are available.

### 2.3 Operational Records.

Hydrological reports detailing rainfall, pool level, and discharge are available at the park office.

### 2.4 Other Investigations.

Monthly inspection reports are available from PennDER files for the period 1960 to 1973. Additional reports were filed in 1940 and 1948, 1974, 1975 and 1976. These reports are available from PennDER and the Bureau of State Parks.

### 2.5 Evaluation.

Sufficient data are available to make a general assessment of the facility.

SECTION 3  
VISUAL INSPECTION

3.1 Observations.

a. The general appearance of the structure and related appurtenances suggest that the facility is in fair condition.

b. Embankment. The pre-construction drawings indicate that the facility was to be constructed of rolled earth or a combination of rolled and semi-hydraulic fill. As-built drawings are not available; however, the surficial appearance of the dam indicates that it was constructed to the lines and grades shown on the contract drawings.

A swampy area is located beyond the toe of the dam over the left portion of the downstream valley (see Photographs 1 and 3). The swamp is situated at the approximate location of the old Shenango River channel as shown on Figure 2 and affects a triangular surface area with a base (along the toe of the dam) of about 500 feet and a depth (measured perpendicular to the base toward the stream channel) of 300 feet. Flow from the swamp is directed via a partially obstructed drainage ditch into the Shenango River just downstream of a gaging station (U.S.G.S. gaging station 300 feet downstream of dam) on the Shenango River outlet channel. Total discharge at this point on the day of inspection was estimated to be less than 5 gpm.

The swamp-like conditions at the toe have existed since the earliest inspection report was filed in 1940. An October 15, 1976, report from the Division of Water Resources Projects, Bureau of Resources, also mentions the problem and attributes it to the phreatic surface in the embankment. The report goes on to say that "in some localized areas, a part of the seepage also appears through the foundation, which could be attributed either to sheet pile crevices or poor contact with the rock bottom". The report concludes that the "marshy and sloughy conditions should be considered as unsafe and adequate protective measures should be provided for the safety of the structure."

No seepage flow could be measured at the downstream toe at the time of inspection. As previously mentioned, however, some flow was discharging into the Shenango River on the downstream side of the swamp.

Pymatuning Dam is reportedly provided with a coarse gravel toe and blind drain below the berm at elevation 984. It is conceivable that the blind drain discharges at the location of the swamp and that the marshy condition results

from poor drainage at this point. It is also possible that some seepage is passing beneath the dam through the glacial soils in the area east of the old Shenango River channel in the area where sheetpiling was not driven to rock.

Another area of seepage was noted to the right (west) of the discharge end of the primary outlet structure (see Photographs 5 and 6). The seepage is passing through in-place soil and is partially diverted via two clay pipe drains which discharge onto the concrete walkway below. Total flow was minimal (could not be measured) and the condition is not considered serious under present conditions.

The downstream face of the dam is provided with a number of slope gutters which direct flow from the roadway (crest) and downstream face to the toe of the dam. When initially constructed, the drains were cobble gutters; however, in more recent years, they have been mantled with concrete (for details of gutter paving, see Figures 1 and 7).

Numerous rodent holes were also noted on the downstream face of the dam.

c. Appurtenant Structures.

1. Spillway. The spillway, spillway sidewalls, apron, and plunge pool all appeared in good condition. Some fine scaling was observed on the ogee-crested weir, the spillway apron, and bridge piers; however, the condition is not considered serious.

Reportedly the spillway has never discharged. Some time after construction, 2 feet high flashboards were added to the ogee section in order to secure additional flood storage capacity (see Photographs 8 and 9). Section 17 of the Maintenance and Operations Manual for the facility states that the flashboards are so constructed that they will fail when they are overtopped by several feet of water.

Water discharging through the spillway is directed into a stilling basin and finally into the downstream Shenango River drainage as shown on Figure 2 and Photograph 11.

2. Primary Outlet Works. The primary outlet works at Pymatuning Reservoir consist of two 6-foot by 8-foot rectangular concrete conduits as shown on Figure 6. Flow through the conduits is regulated via four sluice gates measuring 2-1/2 feet wide by 4 feet high, controlled from the operating room of the gate house (see Figure 11). Stop logs can be placed on the upstream side of the gate chamber

thereby allowing access to the outlet conduit and gate controls. Stop logs had been installed in one of the outlets and this outlet was inspected.

The 6- by 8-foot outlet conduit was in good condition aside from some scaling near the inlet (see Photograph 7). The gate control stems and sluice gate were badly corroded and are reported to be inoperative. According to the park superintendent, a contract to replace the stems, stem guides, stem thrust bolt, and to sandblast and paint the sluice gates has been let and this work is scheduled for the near future.

Recommendations to perform this work can be found in a letter to Mr. Forrey (Director, Bureau of State Parks) as early as 1974. Similar recommendations were made again in 1976.

3. Reservoir Area. The slopes adjoining Pymatuning Reservoir are gentle and partially wooded. No indications of slope distress were observed at the time of inspection.

4. Downstream Channel. The area immediately downstream of Pymatuning Reservoir is characterized as a broad (2,000-foot wide) partially wooded floodplain containing numerous dwellings. The outskirts of the community of Jamestown, Pennsylvania, are located approximately 3,000 feet downstream of the dam. The borough of Greenville is located about 7 miles downstream of the dam. Part of the business center as well as numerous industrial establishments are located adjacent to the river at this point. Suffice to say that a failure of Pymatuning Dam would have a devastating effect on both life and property downstream, possibly effecting hundreds of persons.

Because of the above mentioned considerations, the hazard category is considered "high".

### 3.2 Evaluation.

The outlet works are in need of remedial repair. Some seepage requiring further evaluation was noted at the downstream toe. The seepage has created swampy conditions near the left abutment. This area should be provided with positive drainage and the flow recorded.



## SECTION 4 OPERATIONAL PROCEDURES

### 4.1 Normal Operational Procedures.

An operations manual has been prepared and is used at the facility. The manual lists normal operational procedures as well as flood emergency operation procedures. There are also provisions for normal maintenance and inspection.

### 4.2 Maintenance of the Dam.

The manual lists procedures for inspecting and maintaining the dam including mowing and clearing the dam face, maintaining the slope gutters, and exterminating all burrowing animals.

### 4.3 Maintenance of Operating Facilities.

The manual also contains procedures for inspecting and maintaining the operating facilities. Operating stems are to be kept lubricated and sluice gates painted to prevent corrosion. It is obvious from the present condition of the gates and stems that maintenance has not been thoroughly provided over the years although a certain amount of corrosion should probably be expected on mechanical systems of this age.

### 4.4 Warning Systems.

There are no formal warning systems in effect at the site; however, the park superintendent and the park hydraulic engineer are familiar with the operation of the facility and are available on a full-time basis.

### 4.5 Evaluation.

Periodic inspection reports have been prepared on the facility throughout its history. Maintenance is reportedly provided for the dam and its appurtenances. Although no maintenance reports are available, the Bureau of State Parks is in the process of replacing and repairing portions of the controls on the outlet system.



SECTION 5  
HYDROLOGIC/HYDRAULIC EVALUATION

5.1 Design Data.

No hydrologic or hydraulic data are available.

5.2 Experience Data.

Daily rainfall and reservoir level records are compiled at the facility and kept on file at the Bureau of State Parks offices in Harrisburg, Pennsylvania. The general appearance of the facility indicates adequate past performance. A U.S.G.S. gaging station is located approximately 550 feet downstream of the dam on the Shenango River channel. Hydrometeorological data from this facility is tabulated by the U. S. Weather Bureau.

5.3 Visual Observations.

On the date of the inspection, the general condition of the appurtenant structures of the dam indicate that the facility would likely operate satisfactorily during a flood event. One probable exception to this is the right outlet conduit. Two inoperable gates associated with this conduit make flow regulation impossible. These gates are scheduled for replacement in the near future.

5.4 Overtopping Potential.

The ratio "PMF Peak Flow/Drainage Area" was determined from an empirical curve supplied by the Corps of Engineers, Baltimore District. The curve used was the Ohio River Basin curve. Based on this curve and a drainage area of 158 square miles, Peak PMF  $Q/A = 540$  cfs/sq. mi., and Peak PMF  $Q = 85,320$  cfs. The size category is "large" and the hazard rating "high". Consequently, the SDF is the PMF.

Calculations (see Appendix C) were performed to evaluate the overtopping potential using spillway and storage capacities during the PMF.

The emergency spillway has a maximum discharge capacity of approximately 17,150 cfs assuming that the flashboards atop the spillway crest fail under several feet of head as stated in the Operations and Maintenance Manual. A comparison of maximum spillway discharge (17,150 cfs) with Peak PMF  $Q$

(85,320 cfs) indicates a need for a significant volume of available storage. Based on normal pool elevation 1008 and top of dam elevation 1020, the available storage at Pymatuning Reservoir is calculated to equal approximately 228,000 acre-feet which is greater than the computed storage volume required of 175,000 acre-feet. In fact additional calculations based on storage volume alone indicate that the total inflow to the reservoir during a storm with 26 inches of run-off would be 219,000 acre-feet which is less than the available storage of 228,000 acre-feet. Consequently, it is concluded that the dam is not likely to be overtopped when subjected to a storm of PMF magnitude.

#### 5.5 Spillway Adequacy.

The dam at Pymatuning Reservoir is able to pass and/or contain the inflow resulting from a storm of PMF intensity. As a result, the spillway is deemed adequate.

SECTION 6  
EVALUATION OF STRUCTURAL INTEGRITY

6.1 Visual Observations.

a. Embankment. Based on visual observations, the dam appeared to be in fair condition. A small amount of seepage was observed issuing from two seeps located on the natural hillside to the right of the 6-foot by 8-foot outlet conduits. This condition was not considered serious since flow was minimal and it is possible that it is associated with the drains which appear to be draining the parking lot area on the crest of the embankment between the outlet conduits and the emergency spillway (see Figure 2).

A swampy area is present at the toe of the embankment near the left abutment at the location of the old Shenango River channel. The swampy condition is reported in inspection reports as early as 1940. At the time of inspection, there was no indication of seepage emanating above the toe of the embankment and the condition may simply be the result of poor drainage conditions at the downstream toe. The dam is reportedly provided with a coarse gravel toe and a blind drain; however, the available drawings do not indicate where the drain discharges. A partially obstructed drainage ditch was found just left of the swamp. A small amount of discharge passes through the ditch and enters the Shenango River.

b. Appurtenant Structures.

1. Spillway. The spillway appeared to be in good condition. It appeared that some of the flashboards atop the ogee crest had recently been replaced.

2. Outlet Works. According to available PennDER files, the sluice gate controls which regulate flow through the 6- by 8-foot outlet conduits are in a deteriorated condition. Stop logs had been installed in the right outlet conduit in preparation for repairing the outlet controls. Consequently, we were able to directly observe their corroded condition. Considerable scaling was also observed on the walls of the outlet conduit just downstream of the sluice gates (see Photograph 7).

The gate house and the crane that is used to install the stop logs appeared in good condition.

6.2 Design and Construction Techniques.

Actual design data, design computations, or reports

were not available for any aspect of the facility. It is not known whether the structure was constructed entirely of rolled earth or a combination of rolled earth and semi-hydraulic fill.

### 6.3 Past Performance.

There is no indication that the emergency spillway located on the right abutment has ever discharged. Rainfall records and weir readings taken both upstream and downstream of the dam are available from PennDER files. The facility has apparently functioned well since construction.

### 6.4 Seismic Stability.

The dam is located in Seismic Zone No. 1 and may, therefore, be subjected to minor seismic forces. Considering the swampy area at the toe, the possibility of the embankment being constructed of hydraulic fill and the uncertainty surrounding the engineering properties of the foundation materials, it is possible that even minor dynamic forces could be significant, particularly at high pool levels. However, no design analyses or seismic calculations were made to support this belief.



SECTION 7  
ASSESSMENT AND RECOMMENDATIONS FOR REMEDIAL MEASURES

7.1 Dam Assessment.

a. Safety. The visual inspection, operational history, and available engineering data suggest that the dam is in fair condition. However, the controls for the primary outlet works are in a deteriorated condition and the sluice gates on the right outlet portal are inoperable.

Since the primary purpose of Pymatuning Reservoir is for the conservation of waters entering Pymatuning Swamp and for regulating the flow of water in the Shenango and Beaver Rivers, it is imperative that the primary outlet works at the facility function properly. From a safety standpoint, the problem is less serious considering that the project can store the runoff associated with a storm of PMF magnitude therefore, the spillway system is considered adequate.

According to Bureau of State Parks personnel, they are planning to replace the controls for the outlet works in the near future.

Two areas of seepage were found associated with the embankment. The first was noted to the right of the discharge end of the primary outlet structure (see Photographs 5 and 6). Seepage in this area was minimal and may be associated with tile drains which apparently drain the parking area atop the dam (see Figure 2).

The second was the swampy area at the toe of the dam. The dam is apparently provided with a coarse gravel toe and a blind drain. We were not able to ascertain the location of an outlet for the blind drain and it may be that the swampy condition at the toe persists because of poor drainage conditions or because seepage is passing beneath the sheet-piling in the area of the left abutment where the piling was not driven to rock. In any event the situation requires further evaluation.

b. Adequacy of Information. The data available was considered sufficient to make a Phase I assessment of the facility.

c. Urgency. It is suggested that the recommendations listed below be implemented immediately.

d. Necessity for Additional Investigations. No additional investigations are considered necessary at this time except as noted in Section 7.2.



## 7.2 Recommendations/Remedial Measures.

It is recommended that:

- a. The outlet works be thoroughly evaluated and repairs made which will ensure the operability of the system. Note: According to State Parks personnel, they are in the process of letting a contract to make the necessary reparations.
- b. The deteriorated concrete within the primary outlet conduits be repaired.
- c. The swampy area at the toe of the dam be drained. A weir or weirs should be installed to monitor flow in the region. This information should be transmitted to the Division of Water Resources Projects, Bureau of Resources Programming within the PennDER for their review. The results of the weir readings and the relationship with pool level, rainfall, and seasonal variations should be evaluated by a registered engineer experienced in the design and construction of earth dams. Further investigations, including the installation of observation wells, may be necessary if the seepage is deemed excessive or if abnormal conditions are indicated.
- d. The rodents residing on the downstream face should be exterminated and their burrows filled.
- e. A warning system be developed to allow for the safe evacuation of downstream residents in the event that emergency conditions develop. This plan could easily be incorporated into the "Operations and Maintenance Manual" currently in use.
- f. The dam be inspected on an annual basis to check for hazardous conditions which might develop.

APPENDIX A

CHECK LIST - VISUAL INSPECTION

CHECK LIST  
VISUAL INSPECTION  
PHASE 1

DAM NAME Pymatuning Reservoir COUNTY Crawford STATE Pennsylvania ID # PennDER 20-7 NDI# PA-176  
TYPE OF DAM Earth HAZARD CATEGORY High  
DATE(S) INSPECTION July 28, 1978 WEATHER Partly cloudy TEMPERATURE 75°

POOL ELEVATION AT TIME OF INSPECTION 1007.56 M.S.L. TAILWATER AT TIME OF INSPECTION - M.S.L.

INSPECTION PERSONNEL:

B. M. Mihalcin (GAI) Bureau of State Parks

J. P. Nairn (GAI) Bud Hetrick - Park Superintendent

D. L. Bonk (GAI) Sam Pusateri - Engineer/Hydrologist

B. M. Mihalcin RECORDER

REMARKS OR RECOMMENDATIONS

OBSERVATIONS

VISUAL EXAMINATION OF

SURFACE CRACKS

None observed.

UNUSUAL MOVEMENT OR

CRACKING AT OR BEYOND

THE TOE

None observed.

SLOUCHING OR EROSION OF

EMBANKMENT AND ABUTMENT

SLOPES

None observed.

VERTICAL AND HORIZONTAL

ALIGNMENT OF THE CREST

Good.

RIPRAP FAILURES

None observed. The riprap appears to be a very durable sandstone.



EMBANKMENT

ID # PA-176

SHEET 2

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
-----------------------	--------------	----------------------------

JUNCTION OF EMBANKMENT  
AND ABUTMENT, SPILLWAY  
AND DAM

Good condition

ANY NOTICEABLE SEEPAGE Two small areas of seepage are visible located on the hillside just downstream and to the right of the discharge end of the outlet conduit. The general location of this seepage is approximately 18 feet above the streambed or 10 feet above the top of the concrete wingwall. In addition to these two areas, there is a broad saturated zone located at the toe of the left side of the embankment. This zone is approximately 500 feet wide at the base of the toe but narrows as it extends downstream toward the river.

STAFF GAGE AND RECORDER

DRAINS

Concrete covered masonry gutters divert surface drainage from the downstream face.

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
CRACKING AND SPALLING OF CONCRETE SURFACES IN OUTLET CONDUIT	<p>Minor cracking and deterioration are visible on concrete surfaces. These small isolated areas are not considered to be significant.</p>	
INTAKE STRUCTURE	<p>Stone masonry gate house located approximately 1,400 feet from the left abutment contains the outlet conduit intakes. Two intakes are regulated by 4 sluice gates and a series of wood stop logs.</p>	
OUTLET STRUCTURE	<p>Two parallel concrete box culverts, 6 feet wide by 8 feet high, carry flow from the gate house approximately 300 feet through the embankment to the point of discharge located approximately 1,400 feet from the left abutment. A close inspection of the interior of the conduit reveals moderate scaling across a majority of the interior surface. This condition coupled with minor efflorescence is most prevalent in the upstream 75 feet of the horizontal outlet conduits.</p>	
OUTLET CHANNEL	<p>Curved channel in apparent good condition. Both the left and right embankments are lined with hand-placed riprap for the first few hundred feet. The channel carries discharge from right to left approximately 250 feet downstream of the toe. At a point approximately 900 feet from the discharge end of the outlet conduit is located a concrete weir which can monitor discharge.</p>	
EMERGENCY GATE	<p>Located at the gate house. Reportedly two of the gates are not operable because of corrosion of the gate stems. However, the park superintendent indicated that the gates are scheduled to be replaced in the very near future.</p>	

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
CONCRETE WEIR	An ogee-shaped concrete weir is located at the entrance to the emergency spillway. Atop the weir are located 2 feet of wooden flashboards.	
APPROACH CHANNEL	Curved channel lined with hand-placed riprap extends from the gate house to the spillway located at the right abutment. The channel appears to be in good condition.	
DISCHARGE CHANNEL	Masonry-lined channel just downstream of the weir which discharges into a stilling basin. Flow is then directed into the old Sugar Run channel before converging with the flow from the gated outlet works.	
BRIDGE AND PIERS	Two concrete piers support a bridge which spans the spillway discharge channel. These piers are located immediately downstream of ogee-shaped concrete weir. The bridge serves to connect the west reservoir shore with the roadway situated atop the dam crest which leads to the east reservoir shore.	

GATED SPILLWAY ID # PA-176 SHEET 5

USUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
----------------------	--------------	----------------------------

ONCRETE SILL

N/A.

APPROACH CHANNEL

N/A.

DISCHARGE CHANNEL

N/A.

BRIDGE AND PIERS

N/A.

GATES AND OPERATION  
EQUIPMENT

N/A.



INSTRUMENTATION ID # PA-176

SHEET 6

VISUAL EXAMINATION	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
MONUMENTATION/SURVEYS	None observed.	
OBSERVATION WELLS	None observed.	
WEIRS	See "Outlet Channel, Outlet Works".	
PIEZONETERS	None observed.	
OTHERS		

## VISUAL EXAMINATION OF

## OBSERVATIONS

## REMARKS OR RECOMMENDATIONS

## SLOPES

The lake is a major recreational facility and has an extensively developed shoreline. The slopes are gentle for the most and only partially wooded. Much of the surrounding area is considered swampland.

## SEDIMENTATION

None observed.

DOWNSTREAM CHANNEL ID # PA-176 SHEET 8

VISUAL EXAMINATION OF	OBSERVATIONS	REMARKS OR RECOMMENDATIONS
-----------------------	--------------	----------------------------

CONDITION (OBSTRUCTIONS, DEBARS, ETC.)		
--	--	--

	The downstream channel is a gentle sloping broad floodplain. The first downstream obstruction a roadway bridge which crosses the Shenango River at distance approximately 2 miles downstream of the dam.	
--	--	--

SLOPES		
--------	--	--

Gentle.		
---------	--	--

APPROXIMATE NO. OF HOMES AND POPULATION		
---	--	--

	Extensive development including portions of the communities of Jamestown and Greenville would undoubtedly be affected by a breach of the embankment. Population > 1,000.	
--	--	--

APPENDIX B  
CHECK LIST - ENGINEERING DATA



CHECK LIST  
ENGINEERING DATA  
DESIGN, CONSTRUCTION, OPERATION  
PHASE I

NAME OF DAM Pymatuning Reservoir  
NDI # PA-176  
ID # PennDER# 20-7

SHEET 1

REMARKS

EM

-BUILT DRAWINGS

None available.

REGIONAL VICINITY MAP

See Appendix G.

CONSTRUCTION HISTORY

Partial history available from operating manual supplied by park superintendent.

TYPICAL SECTIONS OF DAM

See Figure 3.

OUTLETS - PLAN See Figures 5, 6, 7, 10, and 11.

- DETAILS See Figures 10 and 11.

- DISCHARGE RATINGS None available.

RAINFALL/RESERVOIR RECORDS

Available at the park office or from the Bureau of State Parks in Harrisburg, Pennsylvania.

DESIGN REPORTS

None available.

EOLOGY REPORTS

None available.

DESIGN COMPUTATIONS

HYDROLOGY & HYDRAULICS

DAM STABILITY

SEEPAGE STUDIES

None available.

MATERIALS INVESTIGATIONS

BORING RECORDS

LABORATORY

FIELD

Boring records available from PennDER files.

POST-CONSTRUCTION SURVEYS OF DAM

None available.

EROSION SOURCES

Not known.

ITEM	REMARKS	ID # PA-176	SHEET 3
------	---------	-------------	---------

#### MONITORING SYSTEMS

None.

#### MODIFICATIONS

Crane added to gate house to mechanically place stop logs.

#### HIGH POOL RECORDS

None available.

#### POST CONSTRUCTION ENGINEERING STUDIES AND REPORTS

Numerous inspection reports available from Penndder files.

#### PRIOR ACCIDENTS OR FAILURE OF DAM DESCRIPTION REPORTS

None.

#### MAINTENANCE OPERATION RECORDS

None available.

ITEM	REMARKS	ID # PA-176	SHEET 4
SPILLWAY PLAN	See Figures 4 and 9.		
SECTIONS	See Figures 4 and 9.		
DETAILS	See Figures 4 and 9.		
OPERATING EQUIPMENT PLANS & DETAILS	See Figures 10 and 11.		



NDI# PA-176

CHECK LIST      ID # PennDER# 20-7

HYDROLOGIC AND HYDRAULIC  
ENGINEERING DATA

DRAINAGE AREA CHARACTERISTICS: 158 sq. miles.

ELEVATION TOP NORMAL POOL (STORAGE CAPACITY): 1008 feet (188,000 acre-feet).

ELEVATION TOP FLOOD CONTROL POOL (STORAGE CAPACITY): 1010 feet (217,000 acre-feet)

ELEVATION MAXIMUM DESIGN POOL: Not known.

ELEVATION TOP DAM: 1020.

SPILLWAY DATA:

- a. Crest Elevation 1008; 1010 (top of flashboards).
- b. Type ogee crest.
- c. Weir Length 114 feet.
- d. Channel Length ≈ 480 feet.
- e. Location Spillover right abutment. (445,000 acre-feet)
- f. Number and Type of Gates none.

OUTLET WORKS:

- a. Type Rectangular concrete box culverts.
- b. Location 1,400 feet right of left abutment.
- c. Entrance Inverts ≈ 973.5.
- d. Exit Inverts 973.
- e. Emergency Draindown Facilities Four sluice gates 2-1/2 by 4 feet  
controlling flow through two 6 by 8 feet  
concrete culverts.

HYDROMETEOROLOGICAL GAGES:

- a. Type Rain gage.
- b. Location Pymatuning park office.
- c. Records at park office.

MAXIMUM NON-DAMAGING DISCHARGE: Not known.

APPENDIX C  
HYDROLOGY AND HYDRAULIC CALCULATIONS

SUBJECT DAM SAFETY INSPECTION  
PYMATUNING RESERVOIR  
BY DLR DATE 7-8-78 PROJ. NO. 78-501-176  
CHKD. BY EJM DATE 8-25-78 SHEET NO. 1 OF 6



### DAM STATISTICS

MAXIMUM HEIGHT OF DAM = 50 FEET (REF 1; pg 3)

DRAINAGE AREA = 158 SQ. MI. " "

STORAGE CAPACITY  $\approx$  217,000 AC-FT @ EL 1010 (REF 1; pg 4)  
 $\approx$  188,000 AC-FT @ EL 1008 " "

### SIZE CLASSIFICATION

DAM SIZE - LARGE (REF 2; TABLE 1)

HAZARD RATING - HIGH (POSSIBLE LOSS OF LIFE > 3)

REQUIRED SDF = PMF (REF 2; TABLE 3)

---

### REFERENCES

- 1 : "OPERATION AND MAINTENANCE MANUAL FOR PYMATUNING RESERVOIR"  
AVAILABLE AT THE PARK OFFICE
- 2 : "RECOMMENDED GUIDELINES FOR SAFETY INSPECTION OF DAMS"  
DEPT. OF THE ARMY - OFFICE OF CHIEF ENGINEER, APPENDIX D
- 3 : STANDARD HANDBOOK FOR CIVIL ENGINEERS  
F.S. MERRITT, MCGRAW-HILL 1976

SUBJECT DAM SAFETY INSPECTION  
PYMATUNING RESERVOIR  
 BY DLB DATE 8-7-78 PROJ. NO. 78-501-176  
 CHKD. BY EJM DATE 8-25-78 SHEET NO. 2 OF 6



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$$\text{PMF (PEAK FLOW)} / \text{AREA} = 540 \text{ CFS/SQ. MI.}$$

(REF: C OF E CURVE,  
OHIO RIVER BASIN)

$$\text{PMF} = (540 \text{ CFS/SQ. MI.}) (158 \text{ SQ. MI.}) = 85,320 \text{ CFS}$$

$$\text{TOTAL TIME OF FLOW} = 100 \text{ HRS}$$

(REF: C OF E CURVE,  
OHIO RIVER BASIN)

#### VOLUME OF INFLOW HYDROGRAPH

$$V = 1/2 (Q_{\text{IMAX}}) (\text{TIME})$$

$$= 1/2 (85,320 \text{ CFS}) (100 \text{ HRS}) (3600 \text{ SEC/HR}) (1 \text{ ACRE} / 43,560 \text{ SQ. FT.})$$

$$= 352,562 \text{ AC-FT}$$

DETERMINE THE AVERAGE RUNOFF REQUIRED TO PRODUCE  
THE ABOVE VOLUME OF INFLOW.

$$(352,562 \text{ AC-FT}) (1 \text{ SQ. MI.} / 640 \text{ ACRES}) (12 \text{ IN/FT}) / (158 \text{ SQ. MI.}) = 41.8 \text{ INCHES}$$

VOLUMES PRODUCED BY RUNOFF IN EXCESS OF 26 INCHES ARE  
TO BE RECALCULATED USING 26 INCHES AS AN UPPER BOUND.

$$(26 \text{ INCHES}) (158 \text{ SQ. MI.}) (640 \text{ ACRES/SQ. MI.}) (1 \text{ FT} / 12 \text{ IN.}) = 219,093 \text{ AC-FT}$$

$$\text{VOLUME OF INFLOW} = 219,093 \text{ AC-FT}$$

NOTE:  $Q_{\text{IMAX}}$  REMAINS CONSTANT

TOTAL TIME OF FLOW DECREASES IN ACCORDANCE WITH THE  
DECREASE IN INFLOW VOLUME

$$\begin{aligned} \text{EQUIVALENT FLOW TIME} &= [(219,093 \text{ AC-FT}) (2) (43,560 \text{ FT}^2/\text{AC})] / (85,320 \text{ CFS}) (3600 \text{ SEC/HR}) \\ &= 62.1 \text{ HRS} \end{aligned}$$



SUBJECT DAM SAFETY INSPECTION  
PYMATUNING RESERVOIR  
BY DLB DATE 9-8-78 PROJ. NO. 78-501-176  
CHKD. BY JPN DATE 9-8-78 SHEET NO. 3 OF 6



CALCULATE AVAILABLE STORAGE

RESERVOIR SURFACE @ NORMAL POOL (EL 1008)  $\approx$  14,528 ACRES  
(REF 1; pg 3 & 4)

RESERVOIR SURFACE @ TOP OF DAM (EL 1020)  $\approx$  23,470 ACRES  
(PLANIMETERED FROM U.S.G.S. 7.5 MINUTE QUADRANGLES)

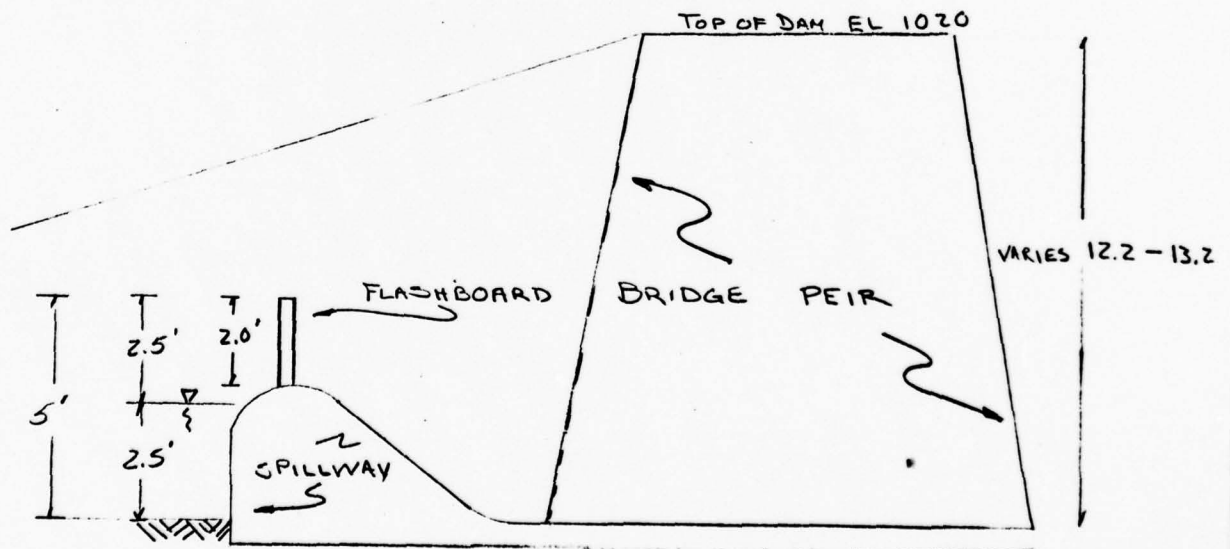
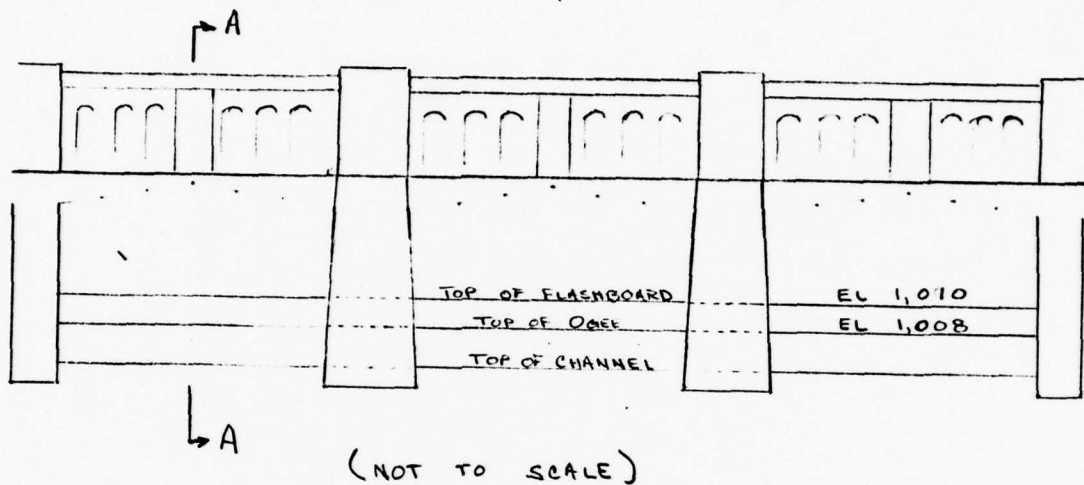
AVAILABLE FREEBOARD = 12 FEET

$$\begin{aligned}\text{STORAGE AVAILABLE} &\approx (12\text{FT})[(23,470 + 14,528)\text{ACRES}/2] = \\ &= 227,988\text{AC-FT}\end{aligned}$$

STORAGE AVAILABLE (227,988 AC-FT) > INFLOW VOLUME (219,093 AC-FT)

SUBJECT DAM SAFETY INSPECTION  
PYMATUNING RESERVOIR  
 BY DLB DATE 8-7-78 PROJ. NO. 78-501-176  
 CHKD. BY EJM DATE 8-25-78 SHEET NO. 4 OF 6

**gai**  
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SECTION A-A (NOT TO SCALE)

NOTE:

ALL ELEVATIONS ARE TAKEN FROM REFERENCE 1, PAGE 3.  
 ALL DIMENSIONS ARE TAKEN FROM FIELD NOTES

SUBJECT DAM SAFETY INSPECTION  
PYMATUNING RESERVOIR  
BY DLB DATE 8-7-78 PROJ. NO. 78-501-17  
CHKD. BY EJM DATE 8-25-78 SHEET NO. 5 OF 6



### SPILLWAY CAPACITY

$$Q = CLH^{3/2}$$

(REF 3: Eq 21-121)

Q = CAPACITY OF WEIR

L = LENGTH OF WEIR = 114'

(REF: FIGURE 9, APPENDIX F)

H = MAXIMUM DISCHARGE HEAD = 12'

(REF 1, pg 3; DIFFERENCE  
IN ELEVATION BETWEEN TOP OF DAM  
AND TOP OF SPILLWAY CREST)

C = COEFFICIENT OF DISCHARGE

$$P/H_D = 3/12 = 0.25$$

$$\therefore C = 3.62$$

(REF 3, FIG 21-67)

$$Q = (3.62)(114')(12')^{3/2}$$

$$Q \approx 17,155 \text{ CFS}$$

NOTE: IT IS ASSUMED THE FLASHBOARDS WILL FAIL UNDER  
HIGH HEADS

SUBJECT DAM SAFETY INSPECTION  
Pymatuning Reservoir  
BY DLB DATE 8-7-78 PROJ. NO. 78-501-176  
CHKD. BY EJM DATE 8-25-78 SHEET NO. 6 OF 6



CONSIDER INFLOW RELATIVE TO BOTH OUTFLOW AND STORAGE  
USING THE SHORT CUT METHOD AS RECOMMENDED BY NAD.

$$P = \frac{\text{MAXIMUM SPILLWAY DISCHARGE}}{\text{PMF PEAK INFLOW}} = \frac{17,155 \text{ CFS (SHEET 4)}}{85,320 \text{ CFS (SHEET 2)}}$$

$$P = 0.20$$

$$(1-P) = \frac{\text{REQUIRED RESERVOIR STORAGE}}{\text{INFLOW VOLUME}} = 0.80$$

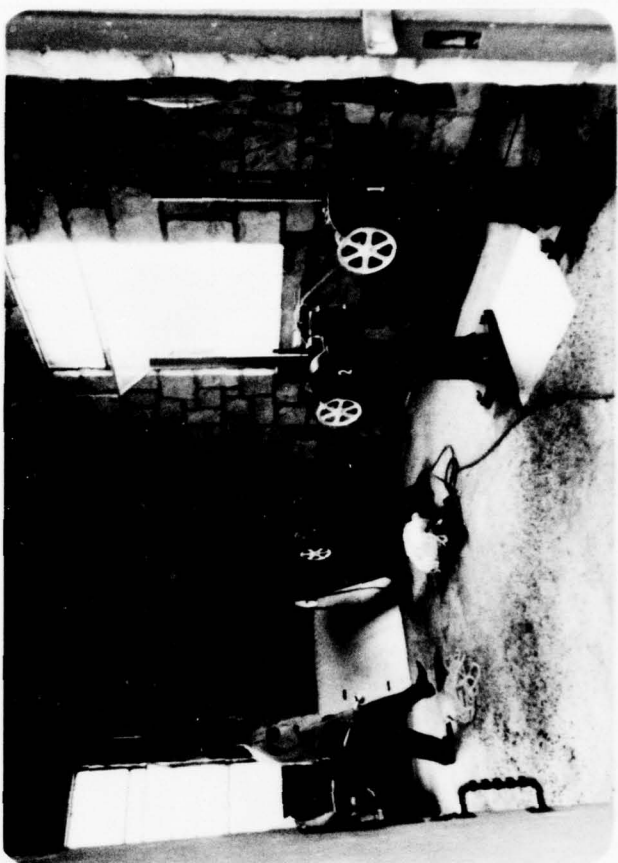
$$\text{VOLUME OF INFLOW} = 219,093 \text{ AC-FT}$$

$$\begin{aligned} \text{REQUIRED RESERVOIR STORAGE} &= (0.80)(219,093 \text{ AC-FT}) = \\ &= 175,274 \text{ AC-FT} \end{aligned}$$

$$\text{STORAGE AVAILABLE (227,988 AC-FT)} > \text{STORAGE REQUIRED (175,274 AC-FT)}$$



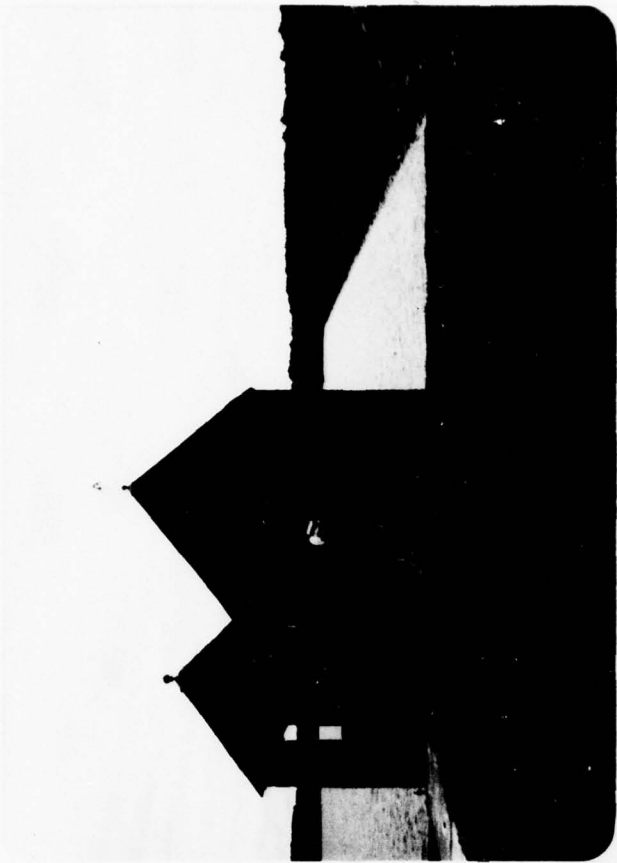
**APPENDIX D**  
**PHOTOGRAPHS**



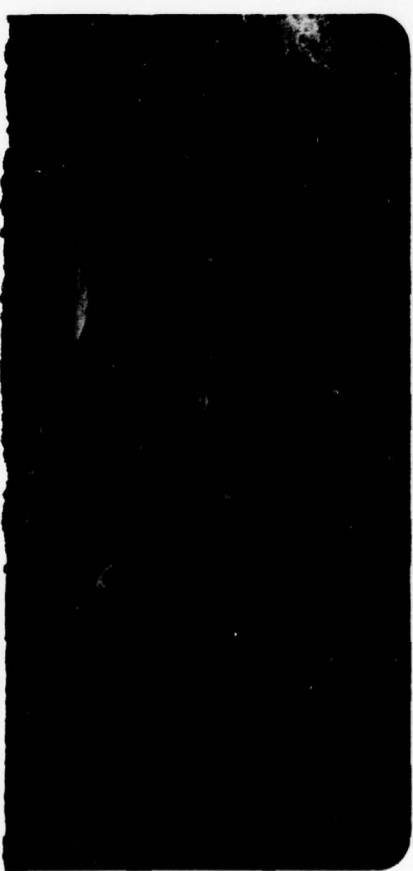
4



3



2



1

PHOTOGRAPH 5

View of the discharge end of the outlet works at Pymatuning Reservoir. Only one portal was discharging at the time of inspection. Note the seepage line in the left portion of the photograph (dark green areas near field team members).

PHOTOGRAPH 6

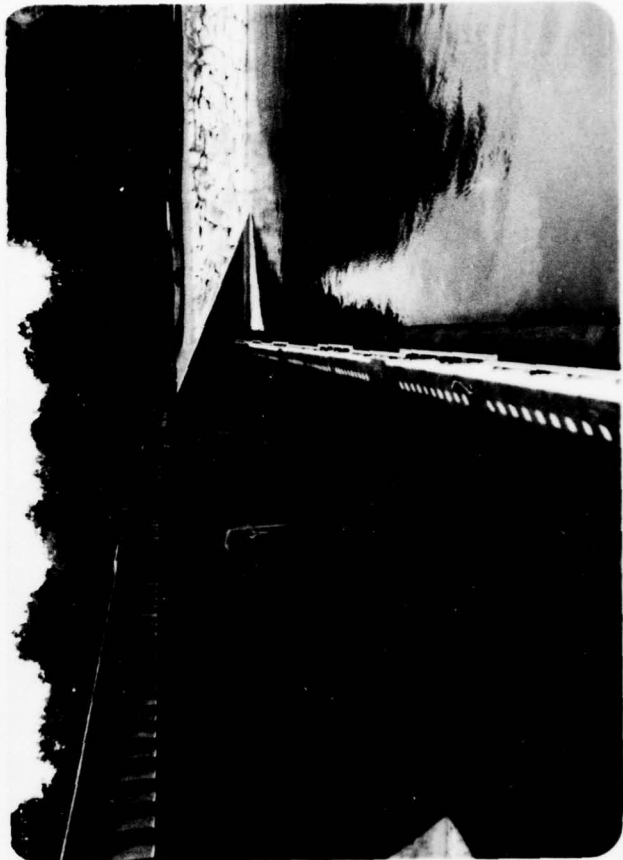
Close-up view of tile drains and seepage near the toe of the dam just to the right of the outlet works.

PHOTOGRAPH 7

Close-up view of scaling within the outlet portal just downstream of the service gates.

PHOTOGRAPH 8

View of the flashboards and ogee-crested spillway located in the right abutment.



8

6



7

5





PHOTOGRAPH 9 View of the masonry-lined approach channel to the spillway.

PHOTOGRAPH 10 View of the spillway channel just downstream of the ogee-crested weir.

PHOTOGRAPH 11 View of the concrete weir located about 500 feet downstream of the Pymatuning spillway plunge pool.

PHOTOGRAPH 12 View of the concrete weir located about 500 feet downstream of the Pymatuning spillway plunge pool.



10



11



9

APPENDIX E

GEOLOGY

The Pymatuning Reservoir Dam is located in the glaciated portion of the Appalachian Plateau Physiographic province of northwestern Pennsylvania. The western-most area of the reservoir extends into northeastern Ohio.

The glacial deposits surrounding the dam and reservoir are chiefly ground moraines of the Kent and Hiram Units of Wisconsin age. Also present are undifferentiated Kames and outwash deposits of various ice advances. Thicknesses of the glacial material are quite variable and are normally 20 to 40 feet but may approach 90 to 100 feet in some areas. The Kent moraine consists of loam to sandy loam till and sand and gravel. The Hiram moraine is chiefly silty clay. The Kame and outwash deposits consist largely of sand and gravel with minor amounts of interbedded silt and clay.

The rock strata underlying the glacial material is of Mississippian and Devonian age. The northern portion of the reservoir pool is underlain by the Devonian age Riceville Formation (Pennsylvania) and the Chagrin Formation (Ohio). The Riceville Formation is composed of interbedded shales, siltstones, and sandstone and the Chagrin is chiefly shale with sandstone interbeds. The southern portion of the reservoir and the dam are underlain by the Pocono Group (Pennsylvania) and Cussewago sandstone (Ohio). The Pocono strata is mostly massive sandstone with some shale.

The structure of the rock strata is characterized by nearly horizontal bedding generally striking northeast and



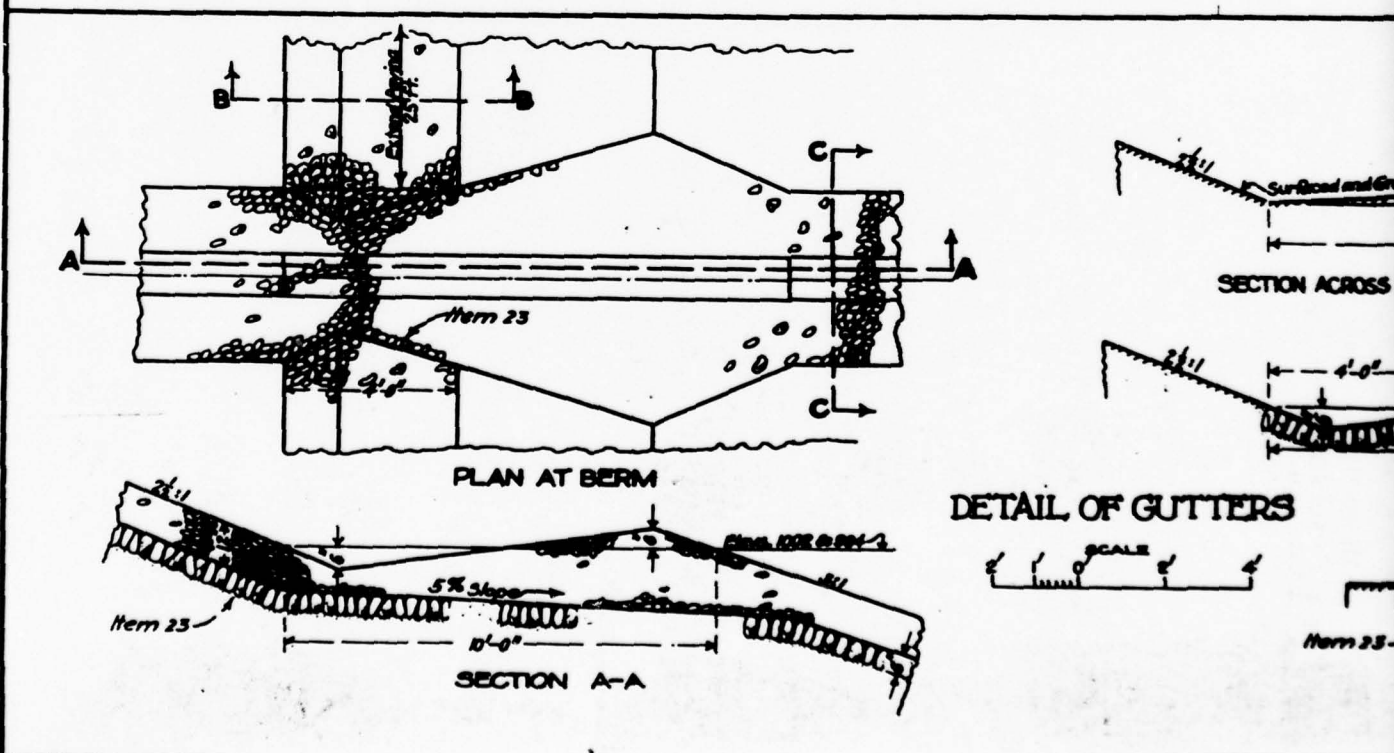
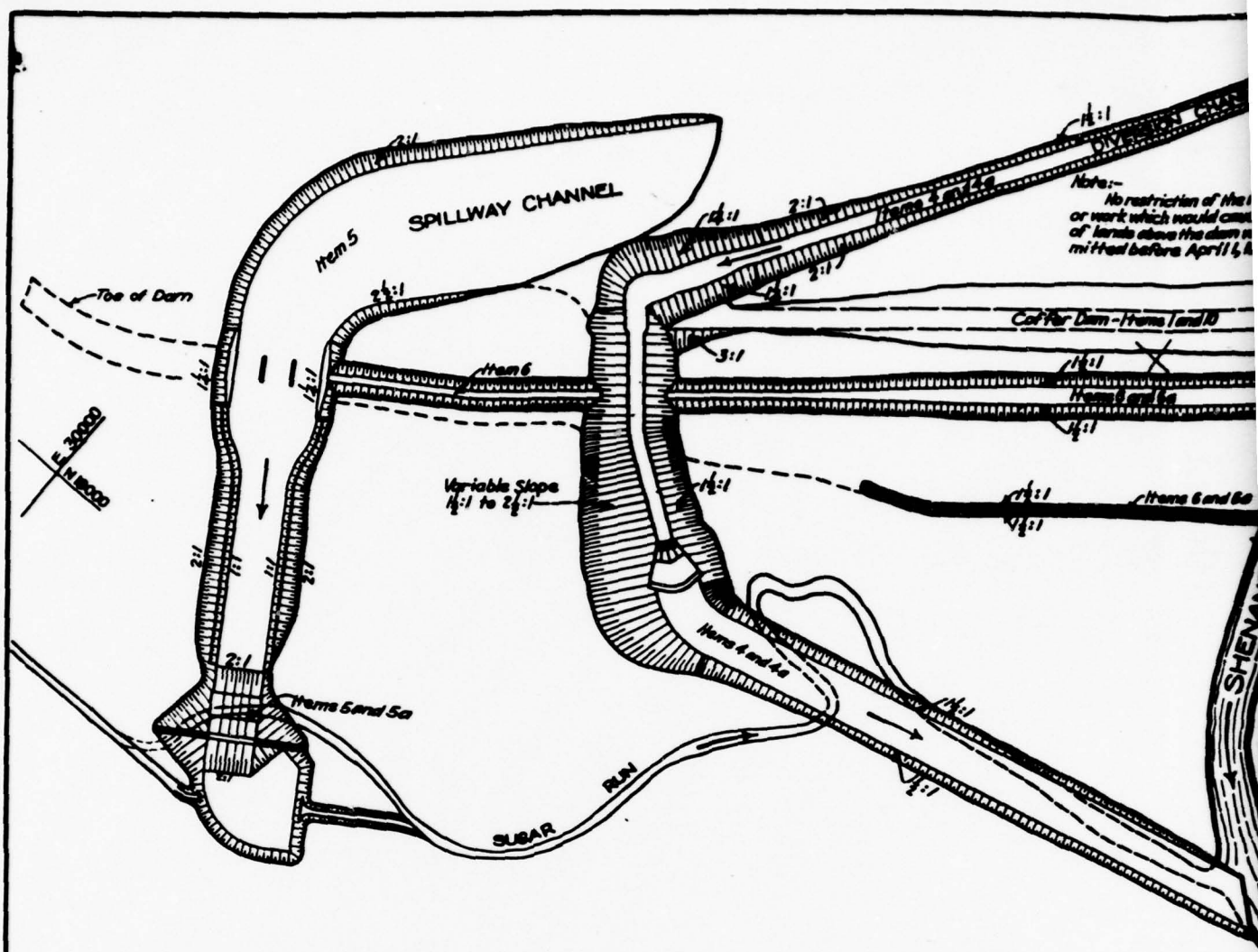
dipping gently to the south or southeast. Jointing within the reservoir area normally has two major sets occurring approximately at right angles. These sets trends N40E and N55W. The set which trends N40E is more prominent and in some of the more resistant sandy layers present structural control.

APPENDIX F

FIGURES

## LIST OF FIGURES

<u>Figure</u>	<u>Description/Title</u>
1	Excavation Plan and Drainage Details
2	General Plan
3	Cross-Section and Profile
4	Spillway
5	Conduits
6	Conduit Entrance and Outlet
7	Grading Details
8	Spillway Bridge
9	Spillway Structure
10	Conduit Entrance
11	Gate House, Plans, Elevation, and Details



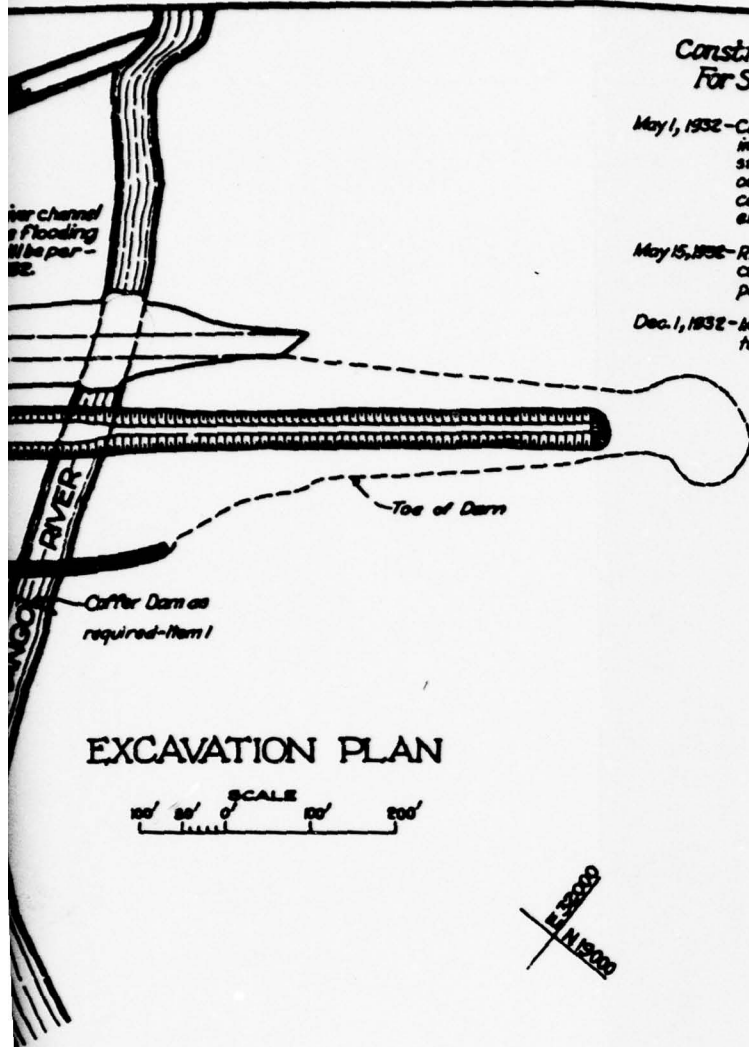


# Construction Schedule For Stream Control

May 1, 1932 - Concrete Conduits completed including entrance and outlet structures and entrance and outlet channels. Coffier dam completed except across river channel.

May 15, 1932 - River flow diverted through conduits and coffer dam completed to elevation 985.

Dec. 1, 1932 - Main embankment completed to elevation 1002.

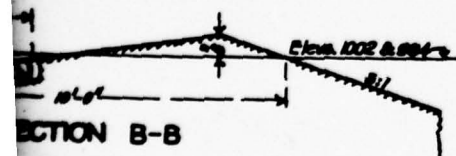


## EXCAVATION PLAN

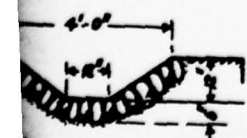
SCALE  
100' 200'



## SECTION A-A



## SECTION B-B



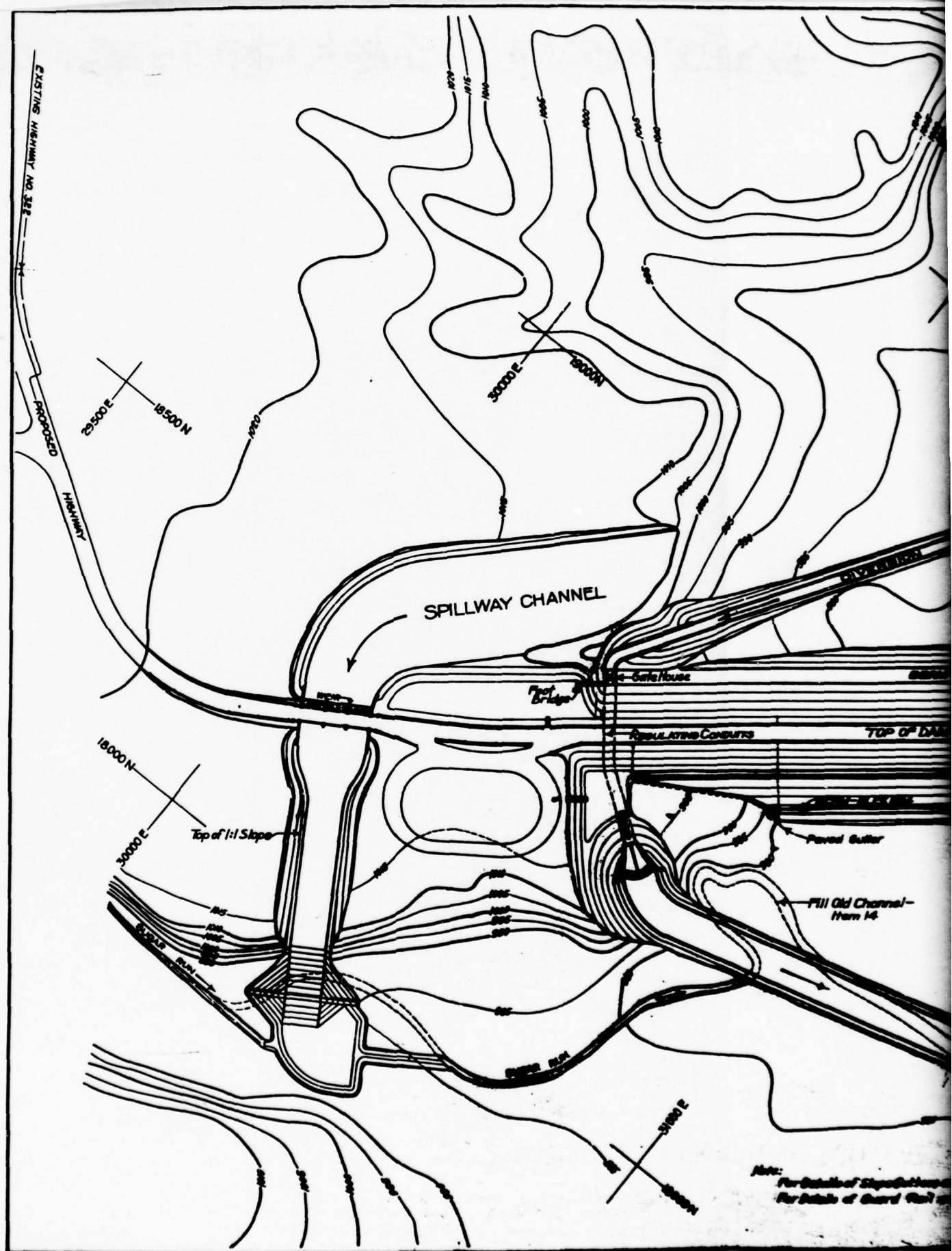
## SECTION C-C

CONTRACTORS OF PLYMOUTH  
WATER AND POWER RESOURCES BOARD

## PYMATUNING DAM EXCAVATION PLAN AND DRAINAGE DETAILS

SCALE AS SHOWN  
CONTRACT DRAWING NO. 6

*[Signatures]*  
JULY 8 1932  
JULY 8 1932  
JULY 8 1932



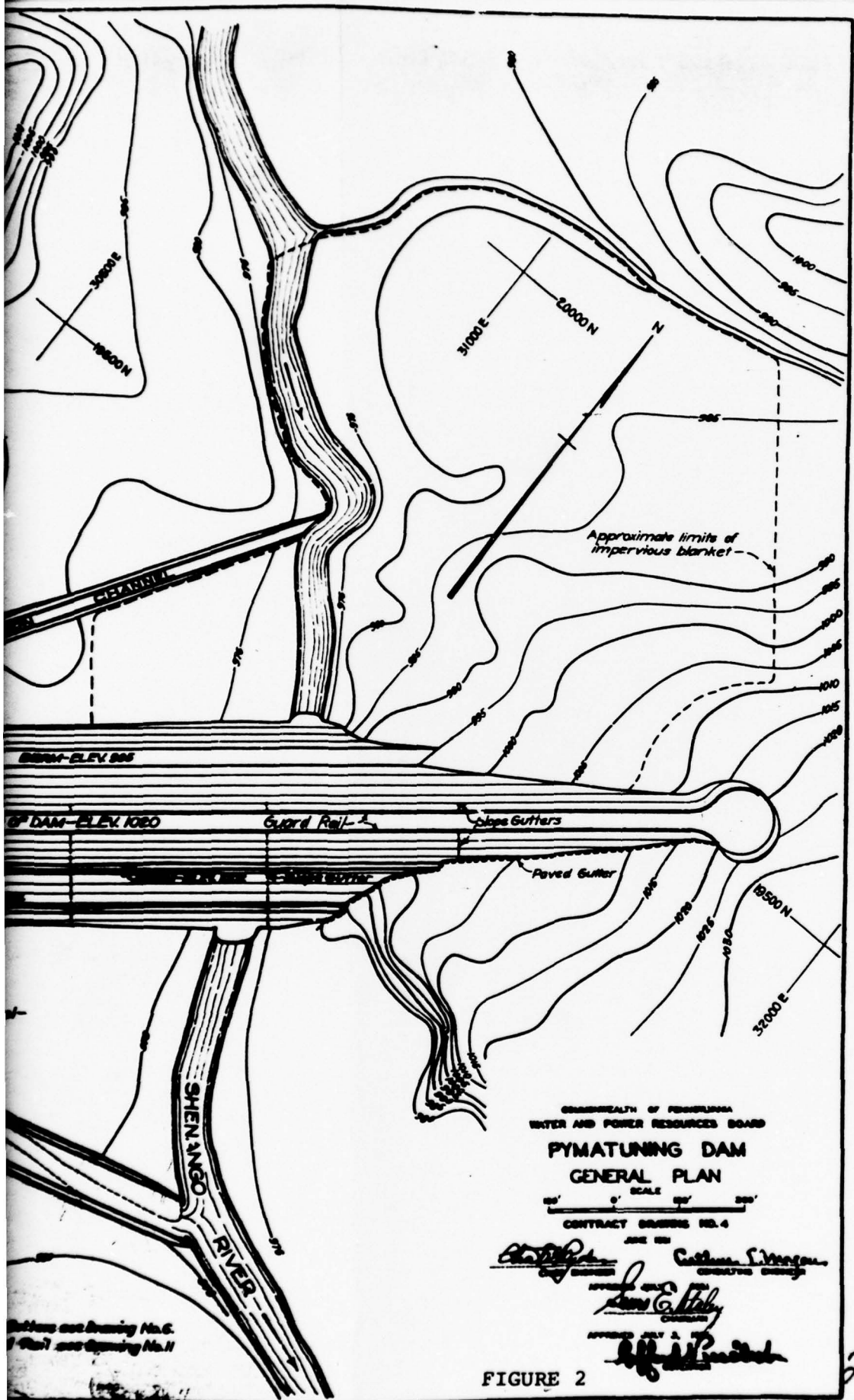
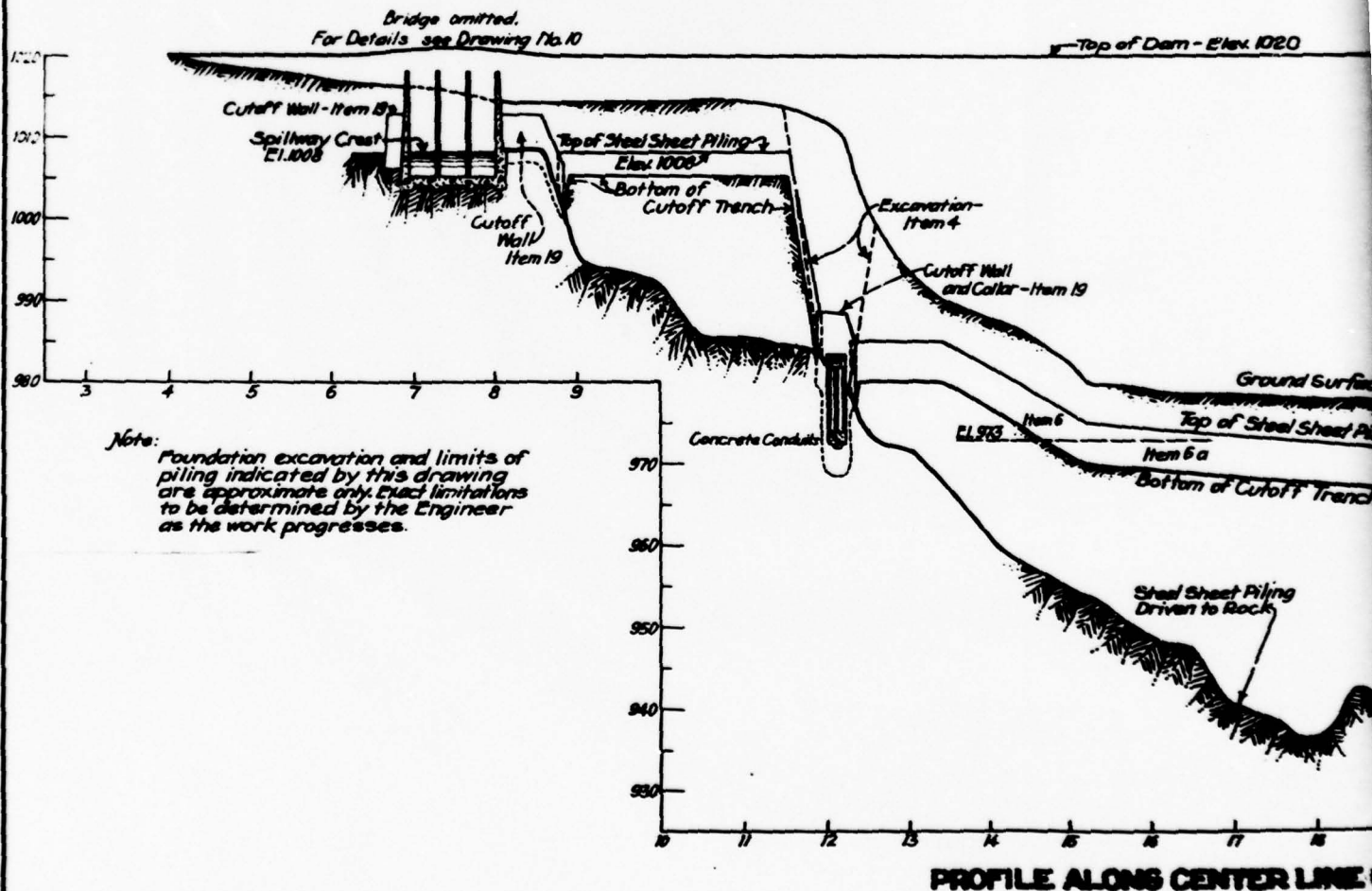
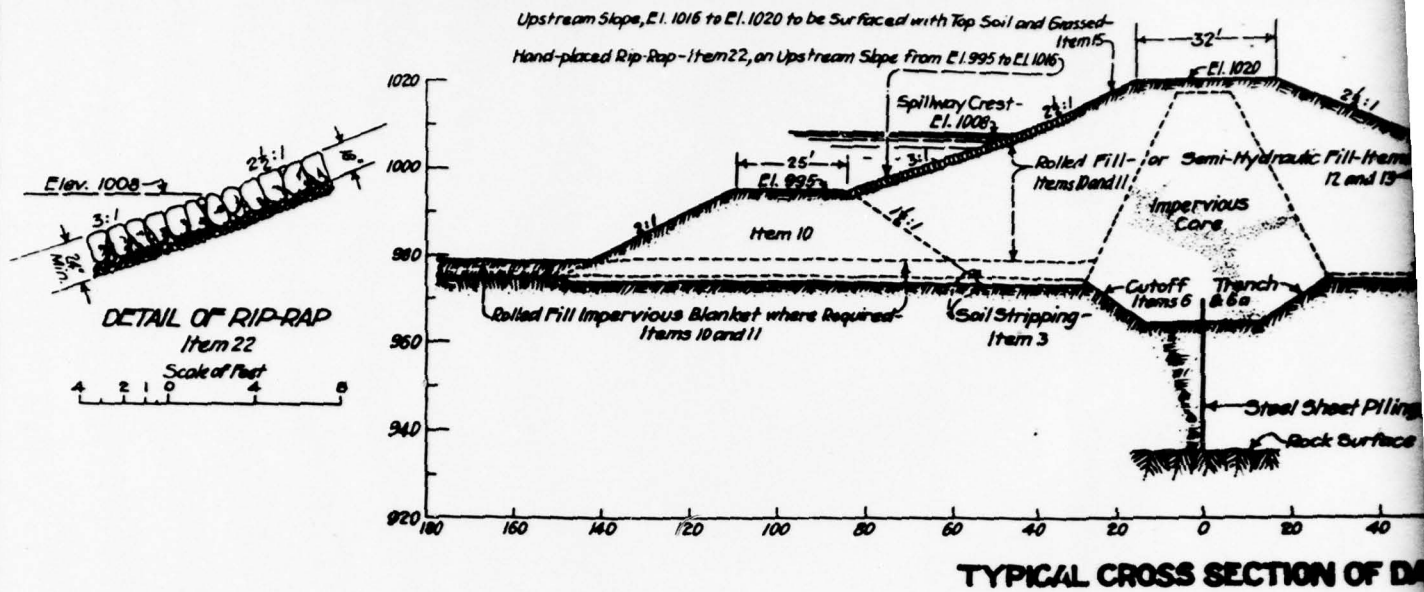
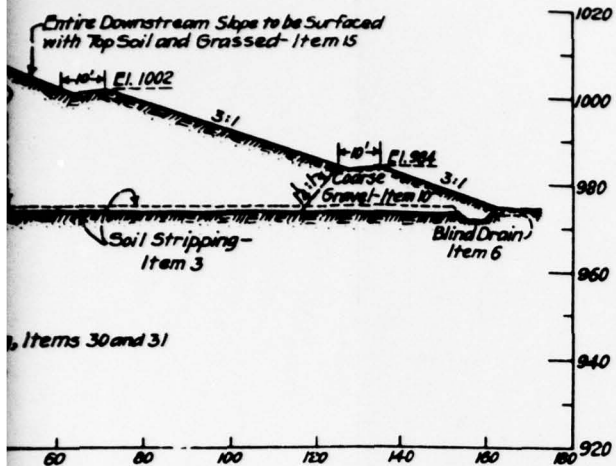


FIGURE 2







AM

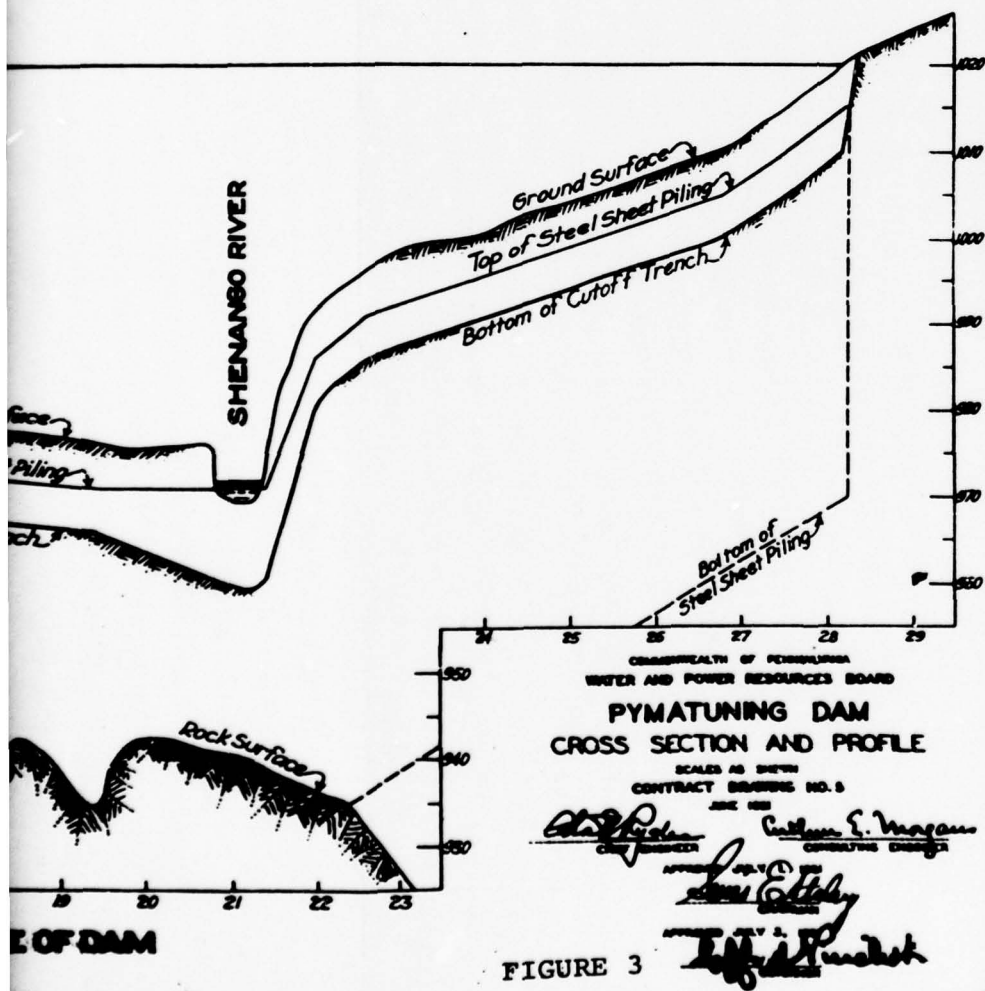
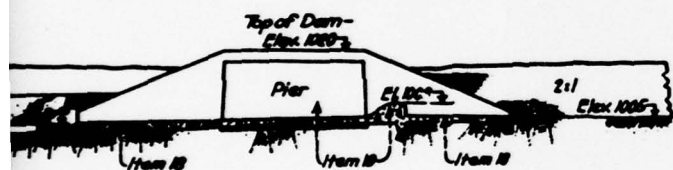
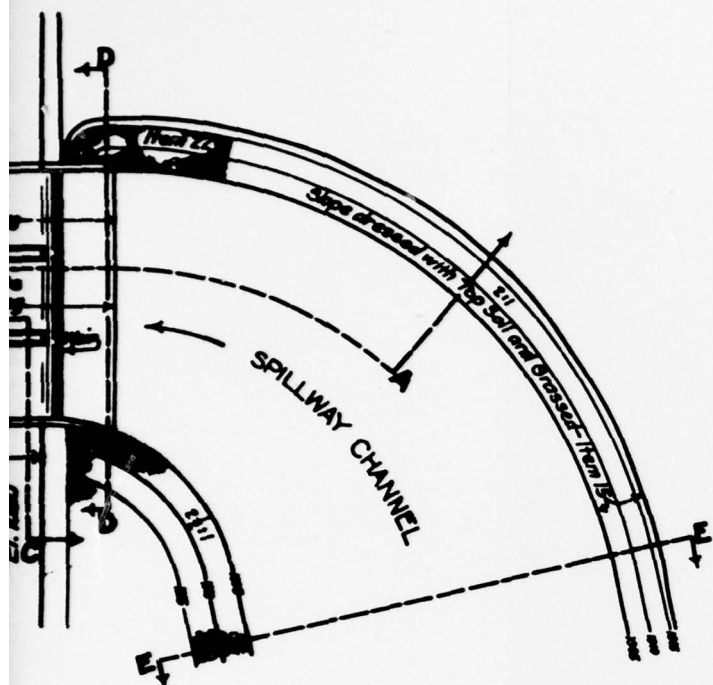


FIGURE 3

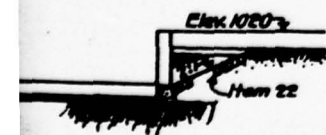
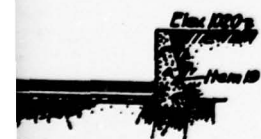
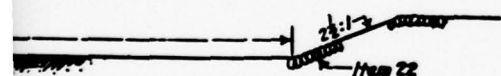
2







A-A



COMMONWEALTH OF PENNSYLVANIA  
WATER AND POWER RESOURCES BOARD  
**PYMATUNING DAM**

**SPILLWAY**

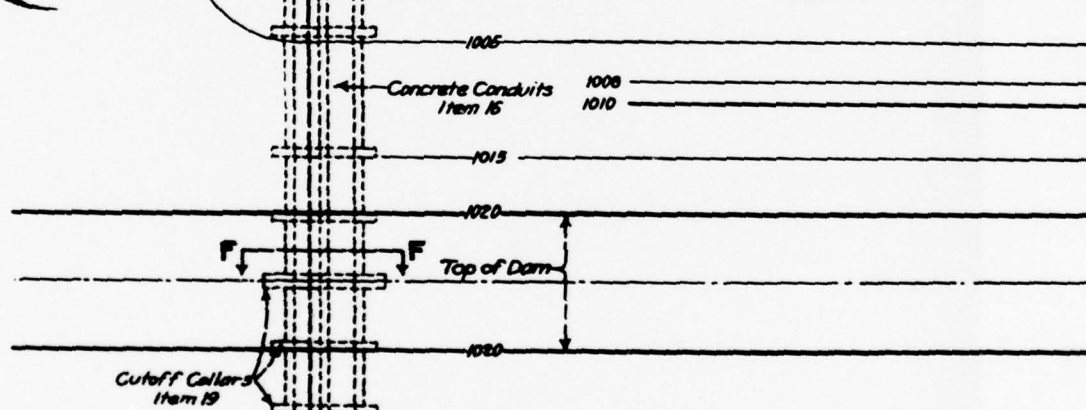
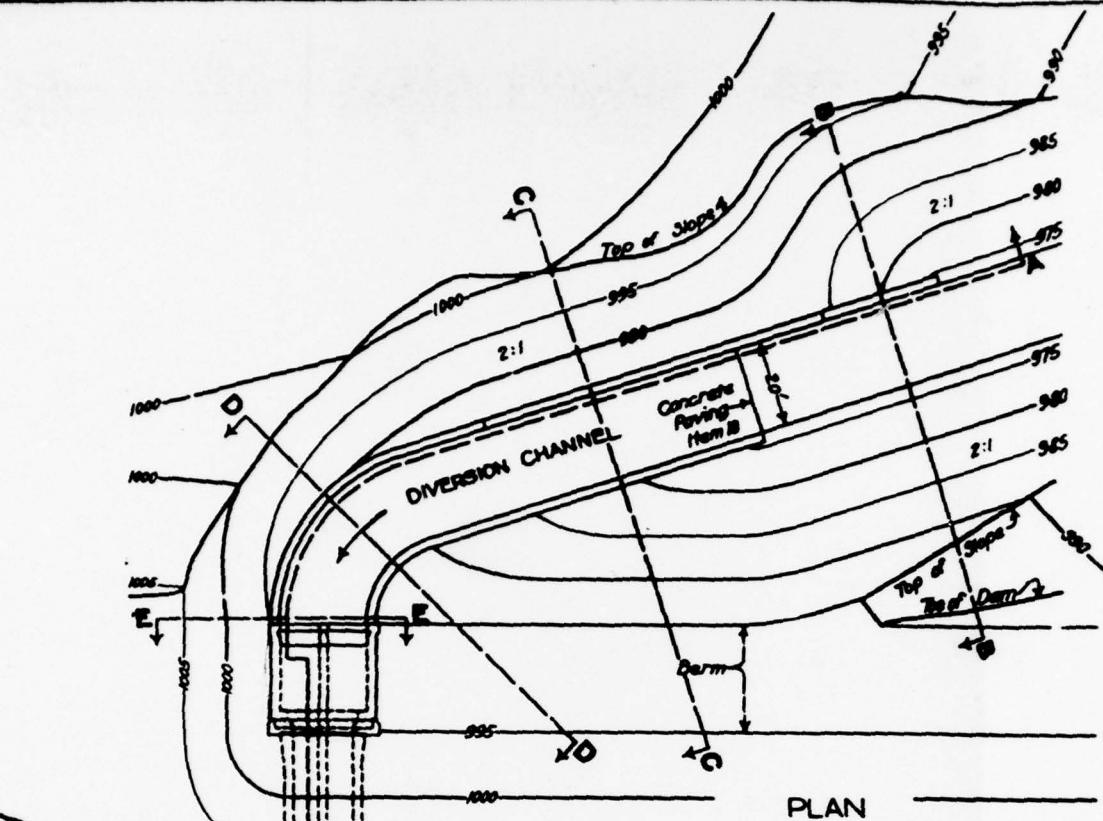
SCALE AS SHOWN  
CONTRACT DRAWING NO. 7

JUNE 1952

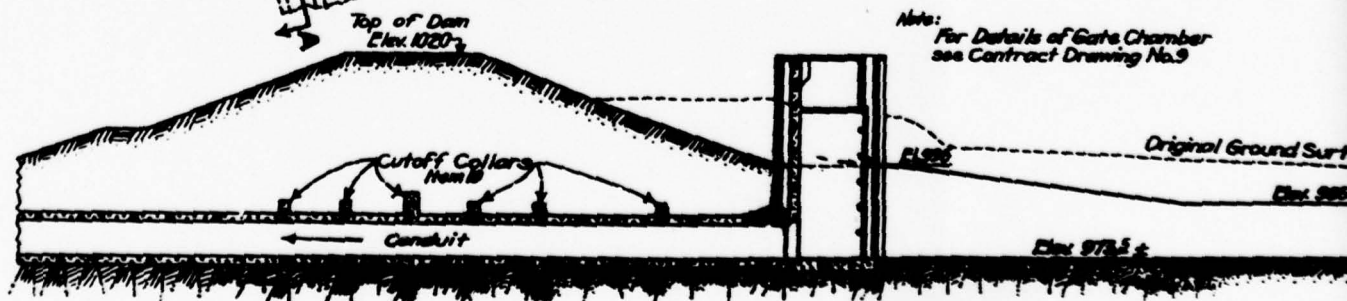
*[Signature]*  
Chief Engineer  
*[Signature]*  
Consulting Engineer  
*[Signature]*  
Surveyor  
*[Signature]*  
Inspector

FIGURE 4

2

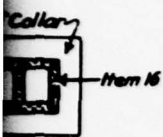


Note:  
For Detail of Conduit Outlet  
See Contract Drawing No. 9



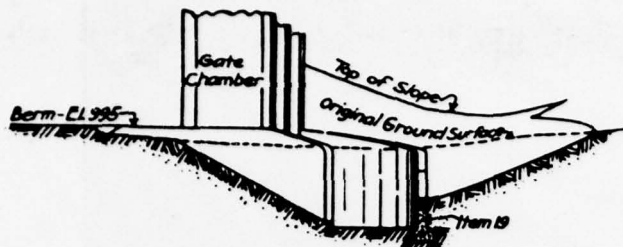
Note:  
For Details of Gate Chamber  
See Contract Drawing No. 9

Item 19  
SECT  
Note:  
Conduit  
Concrete  
Reinforced

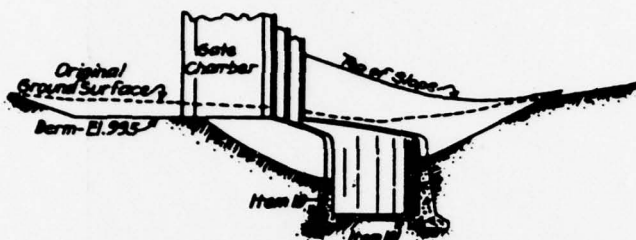


ON F-F

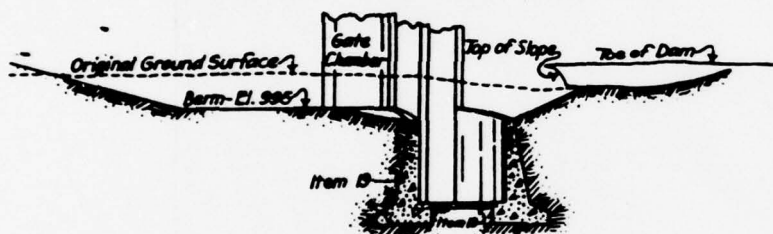
8 ft. x 8 ft.  
Bells 24 inches,  
as Required.  
see 21



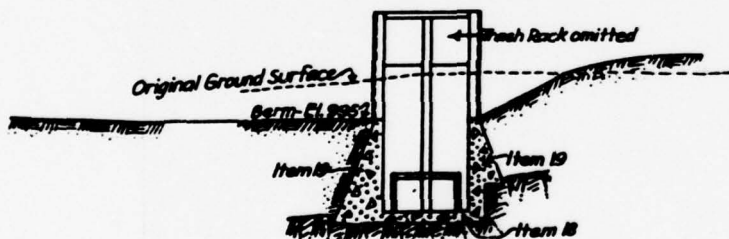
SECTION B-B



SECTION C-C



SECTION D-D



SECTION E-E

COMMONWEALTH OF PENNSYLVANIA  
WATER AND POWER RESOURCES BOARD

**PYMATUNING DAM  
CONDUITS**

SCALE

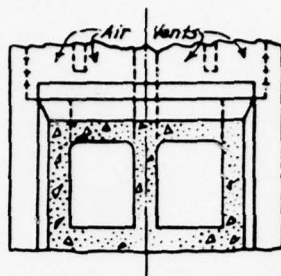
CONTRACT DRAWING NO. 6

*[Signatures]*  
JUL 2, 1971  
*[Signature]*

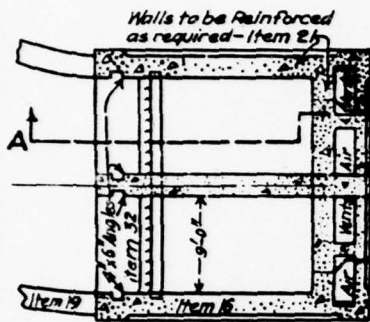
FIGURE 5

2



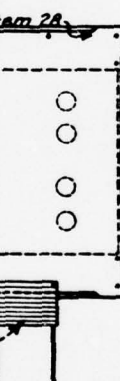
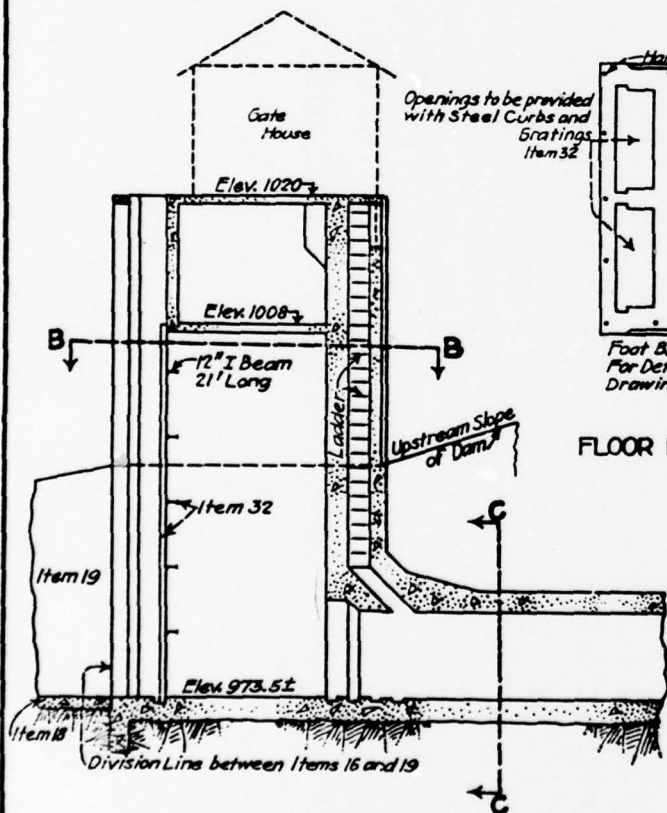
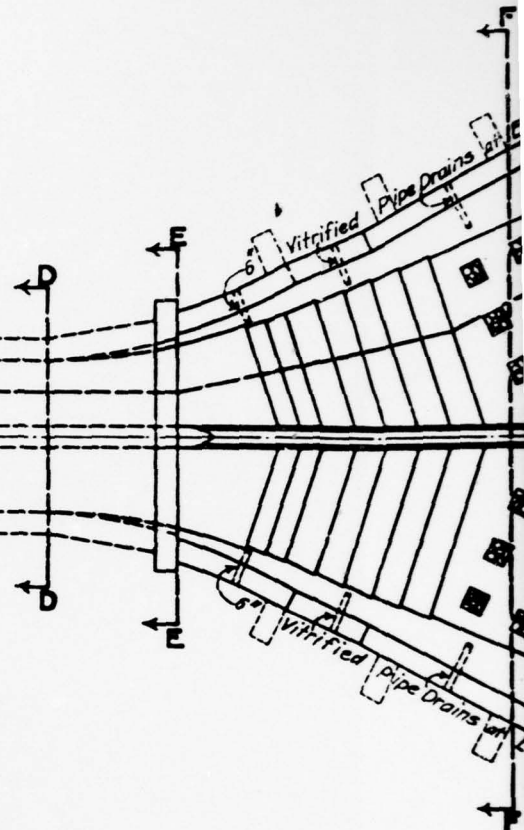


SECTION C-C

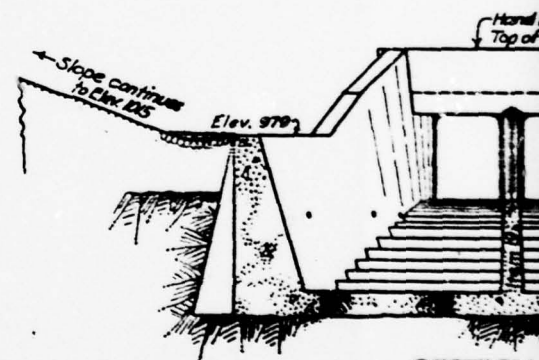


SECTION B-B

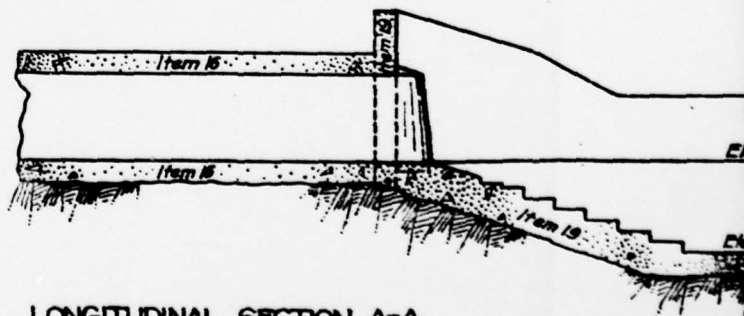
PLAN



FLOOR PLAN AT ELEV. 1020



SECTION



LONGITUDINAL SECTION A-A



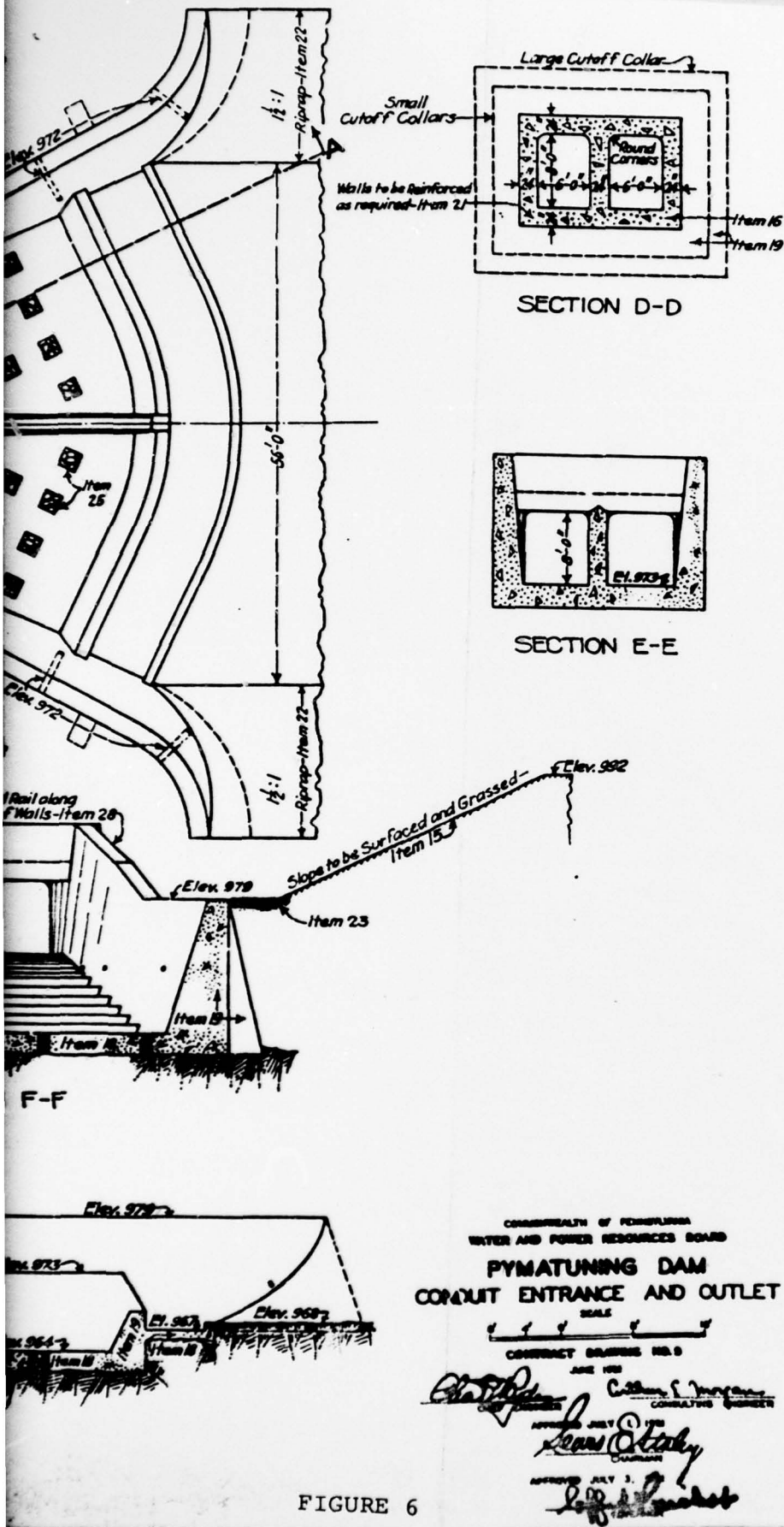
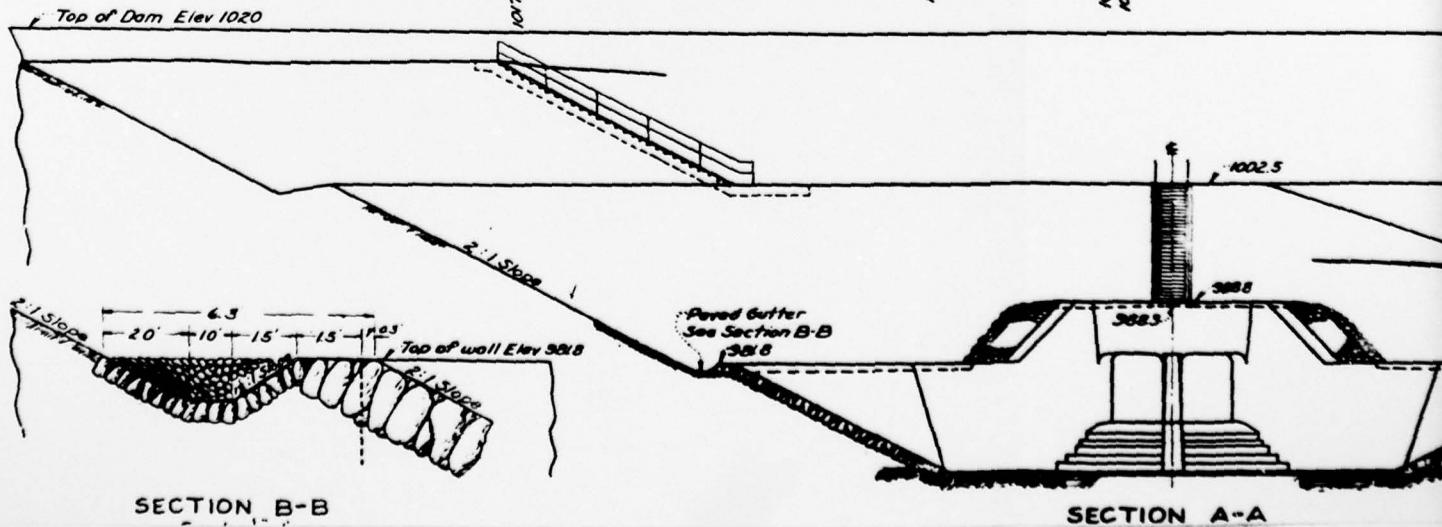
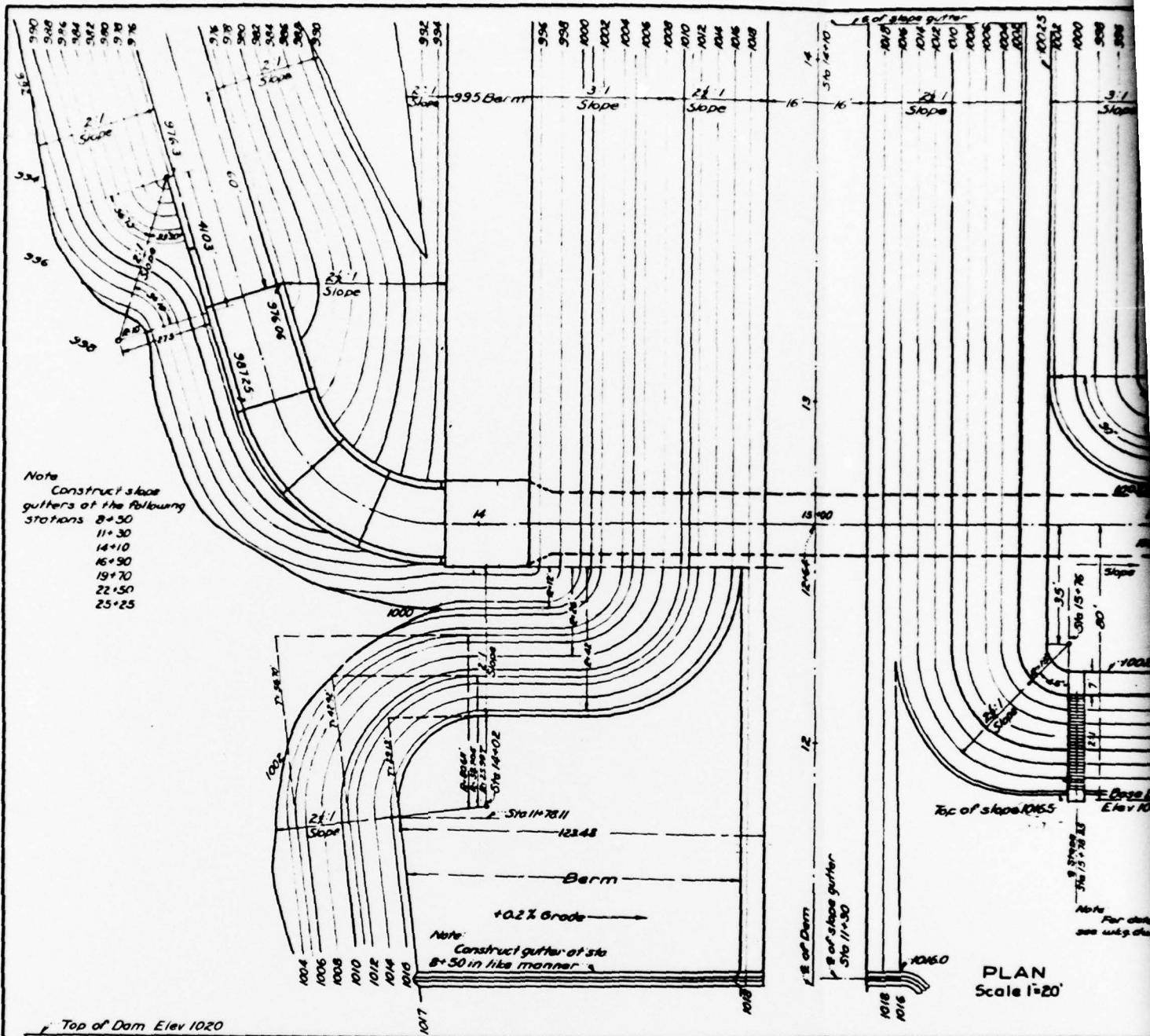
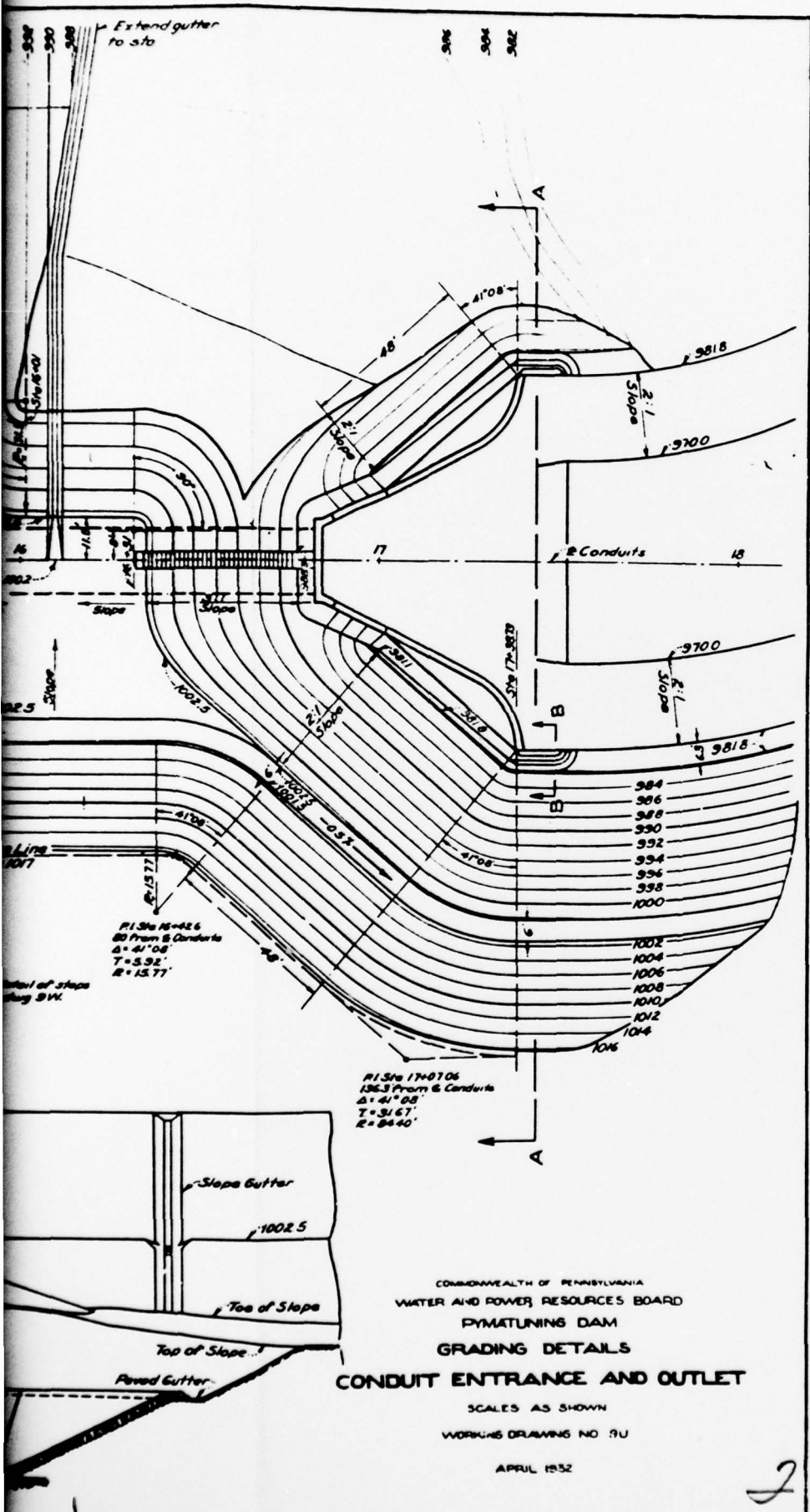


FIGURE 6

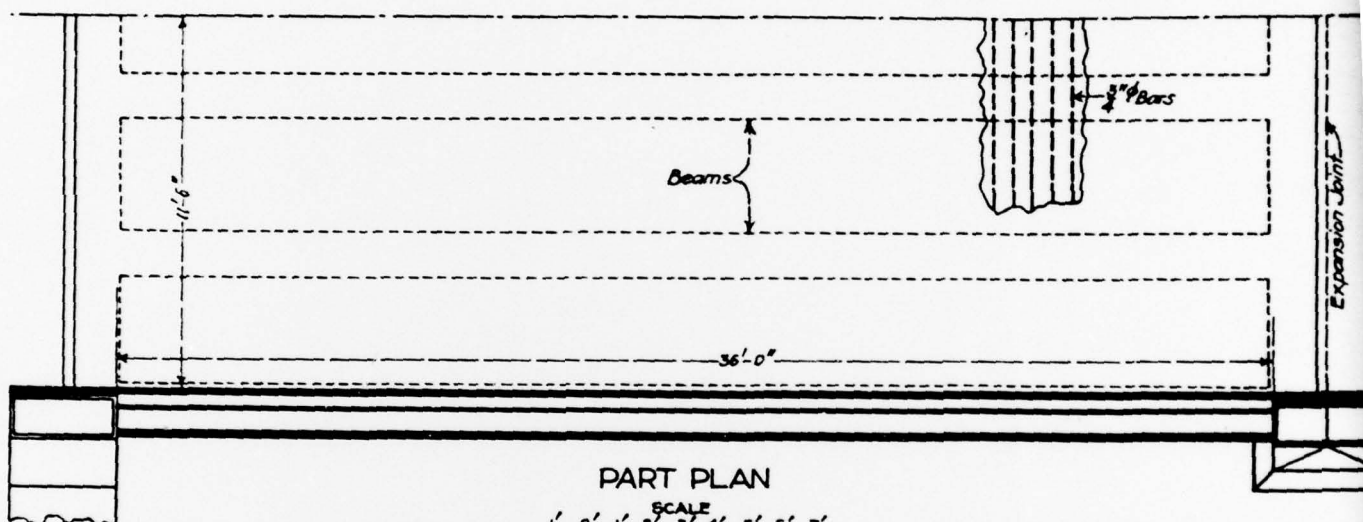
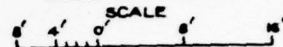




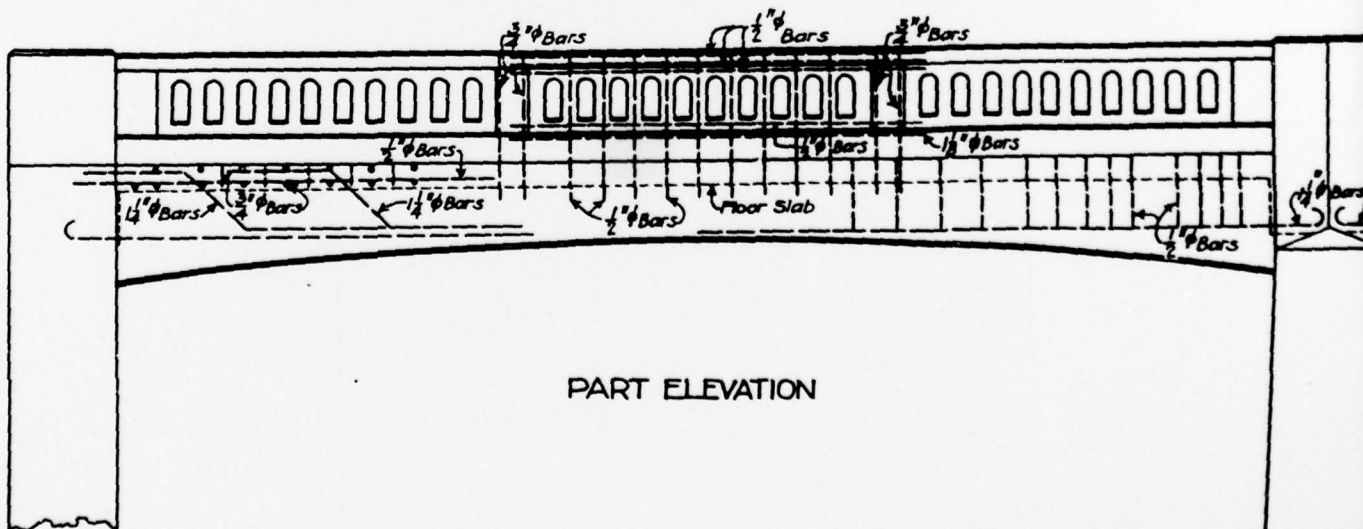
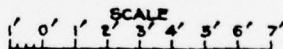


Weir Crest Elev. 1008

ELEVATION

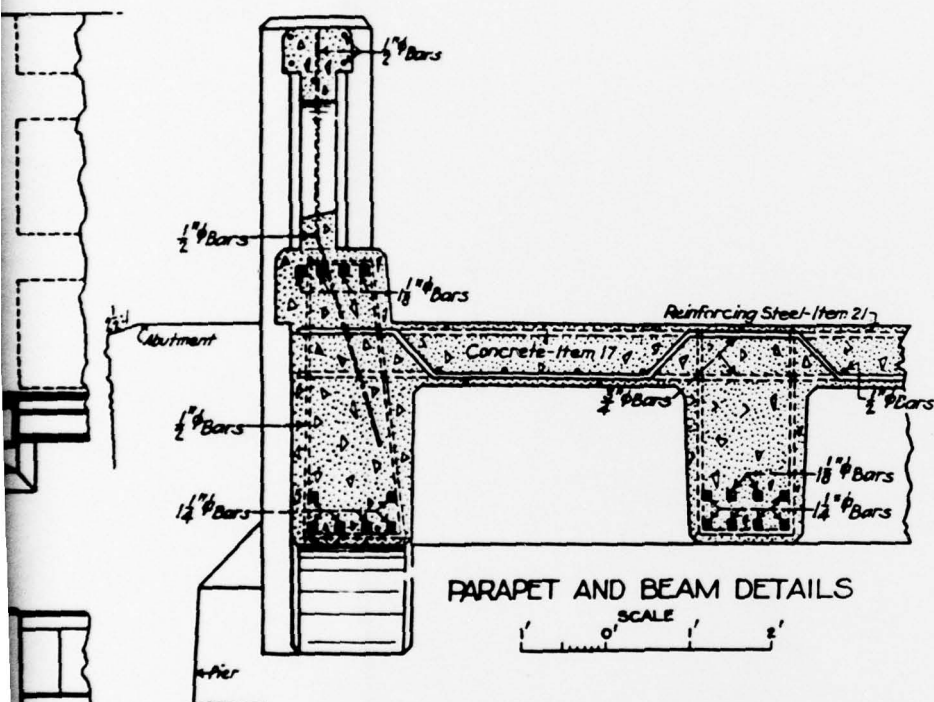


PART PLAN



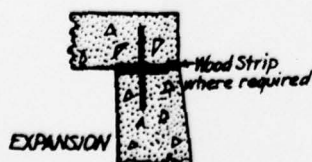
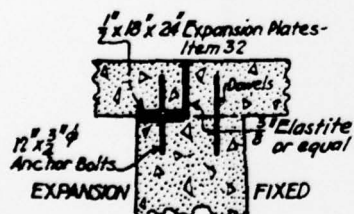
PART ELEVATION





## PARAPET AND BEAM DETAILS

SCALE



## EXPANSION DETAILS

COMMONWEALTH OF PENNSYLVANIA  
WATER AND POWER RESOURCES BOARD

**PYMATUNING DAM  
SPILLWAY BRIDGE**

SCALES AS SHOWN  
 CONTRACT DRAWING NO. 10  
 JUNE 1921

**JUNE 1934**

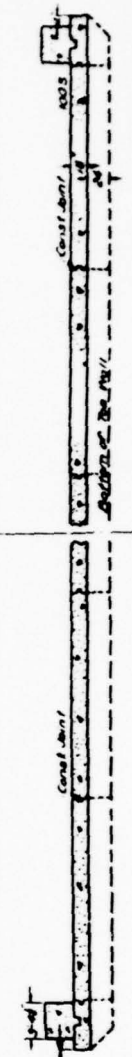
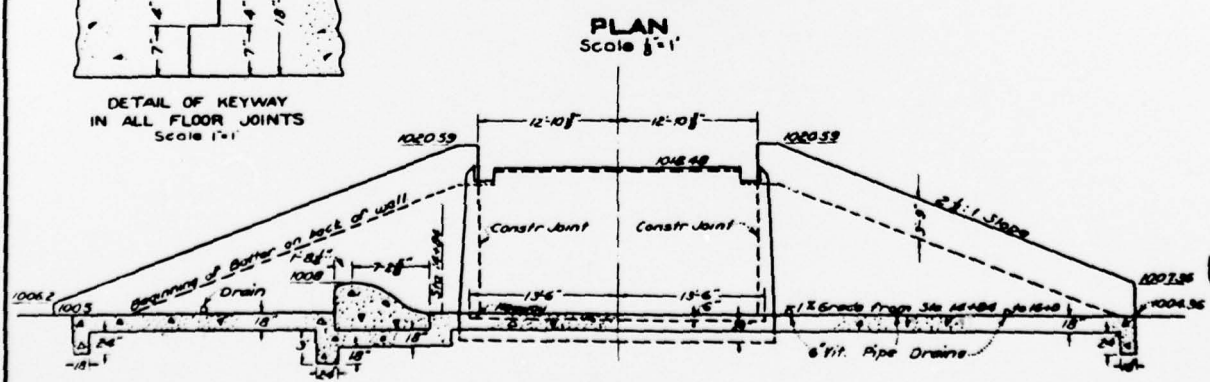
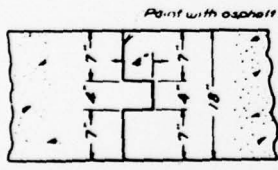
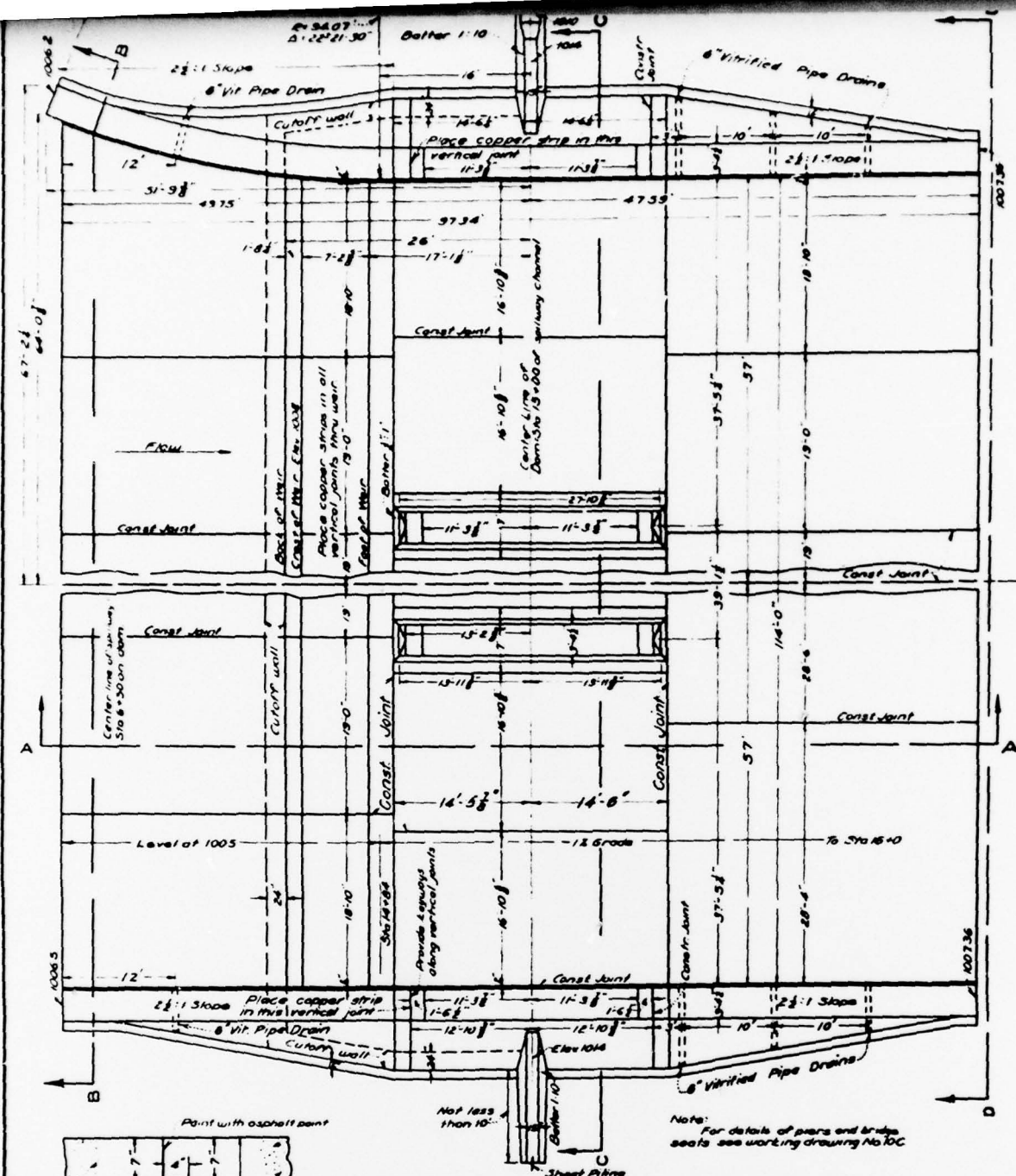
**GETTLE**  
ONE SPINER

*Catherine E. Morgan*  
CONSULTING ENGINEER

APPROVED MAY 1 1971  
James E. Baker  
CHAIRMAN

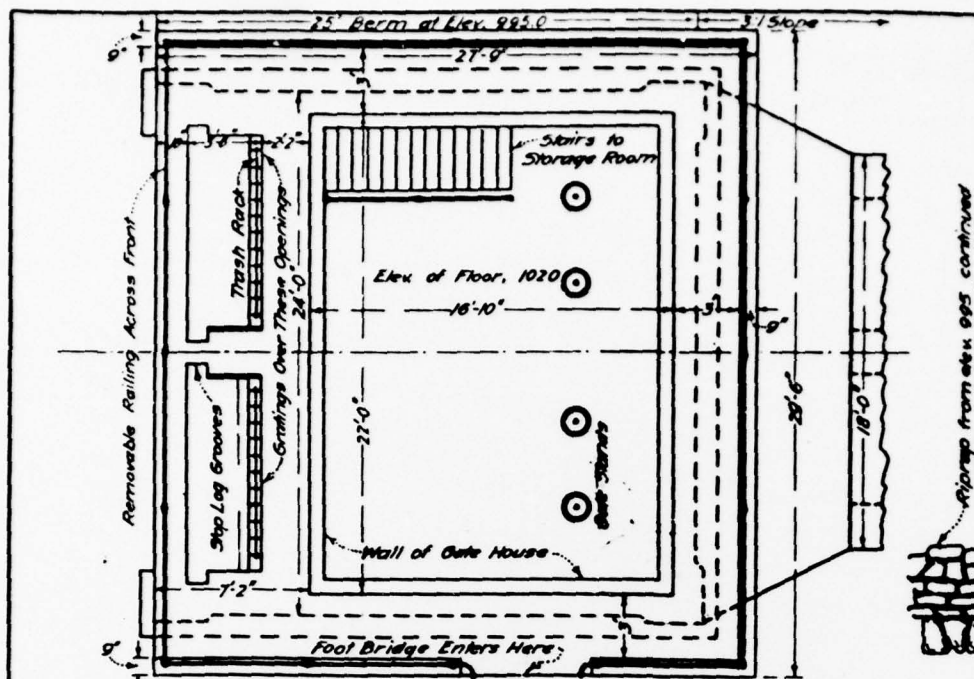
APPROVED JULY 1, 1971  
*[Signature]*  
SPECIAL AGENT



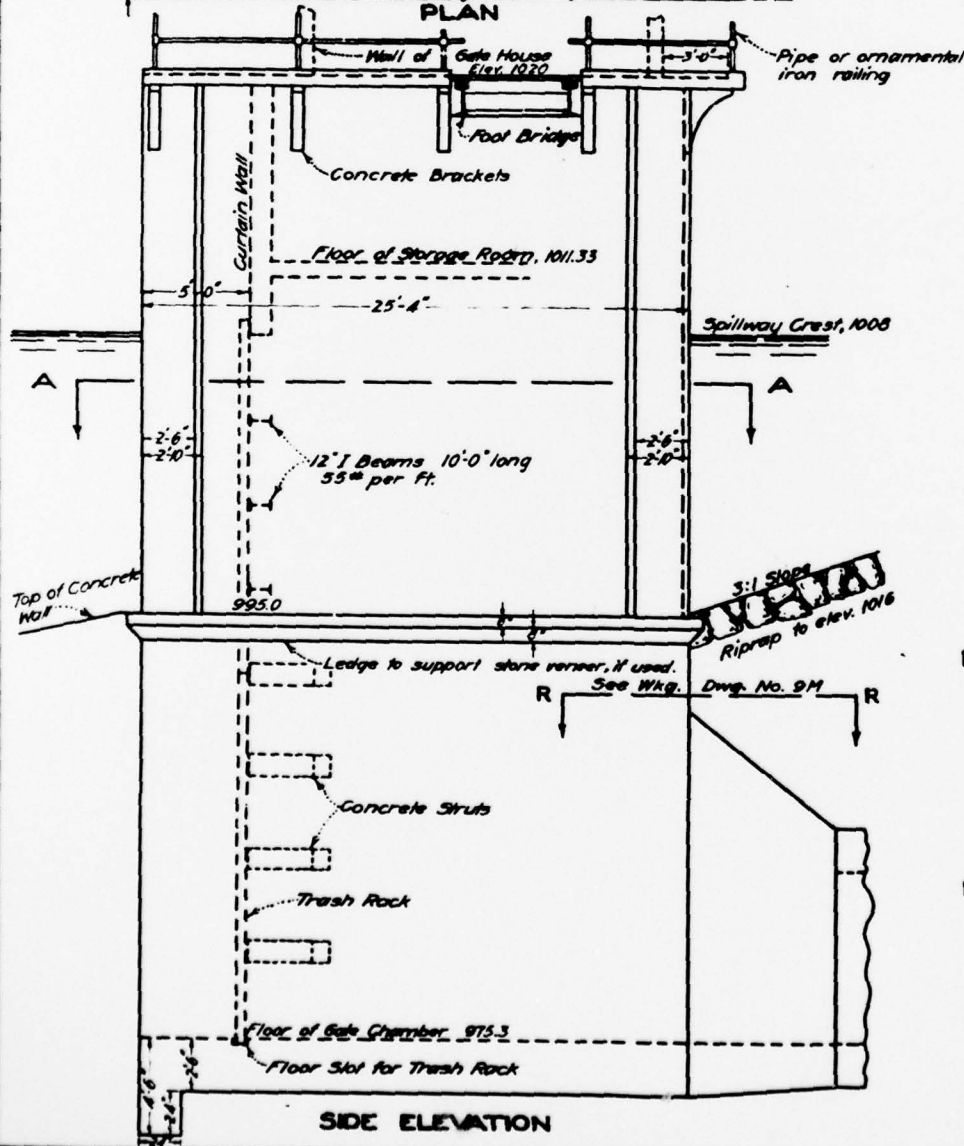


SECTION B-B

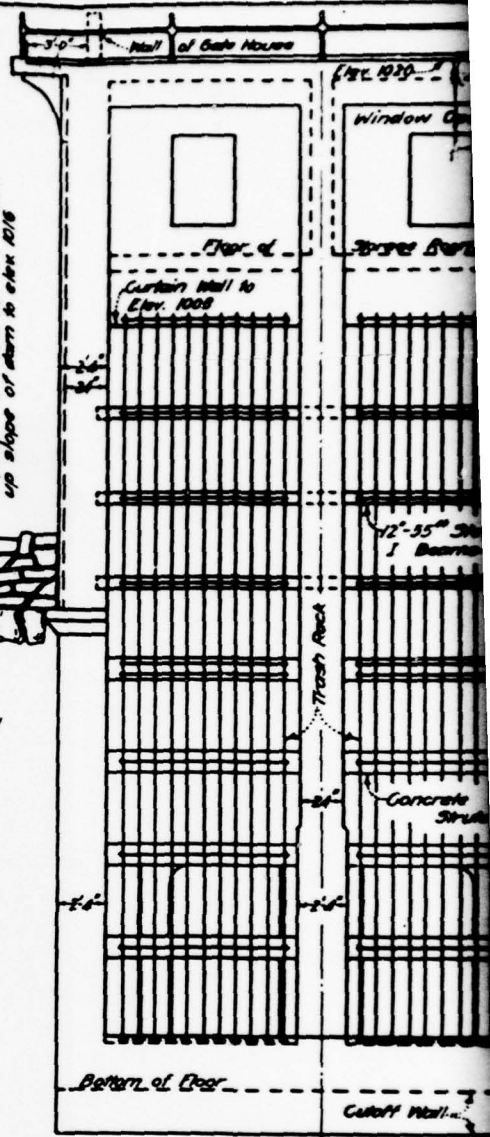




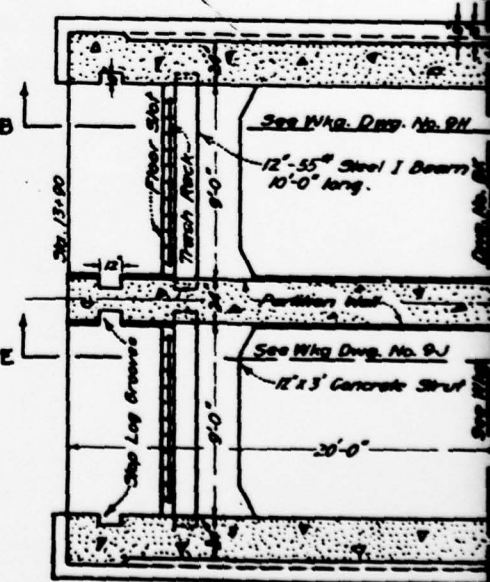
PLAN



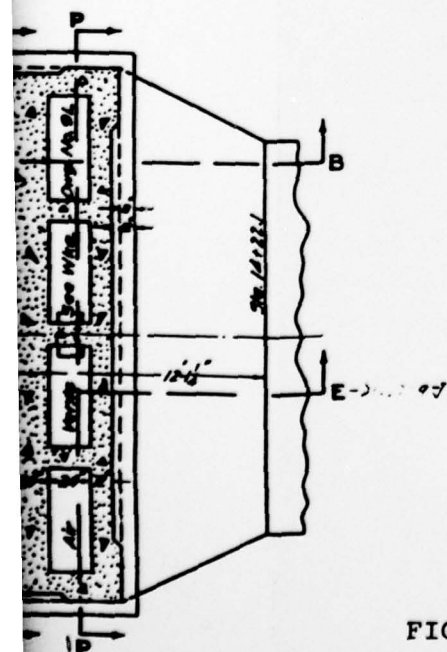
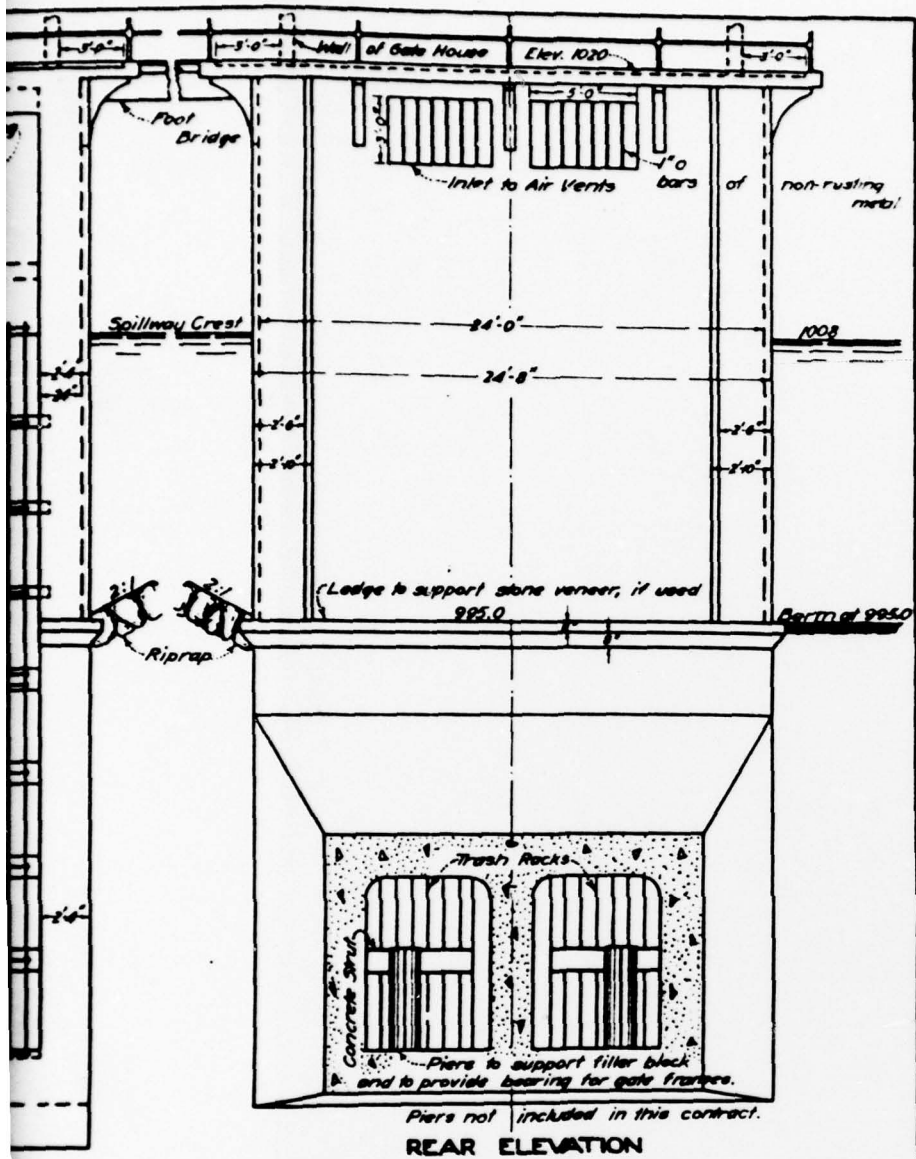
SIDE ELEVATION



FRONT ELEVATION



SECTION A-A



# PYMATUNING DAM CONDUIT ENTRANCE

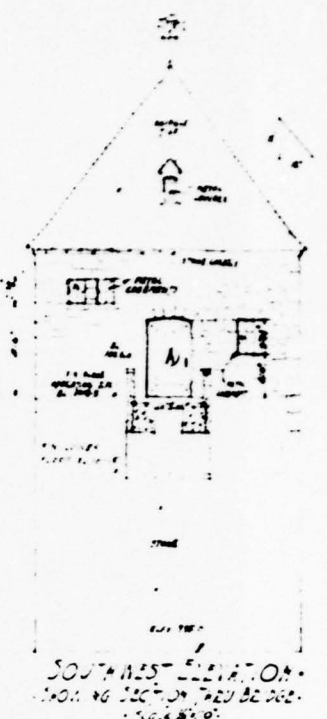
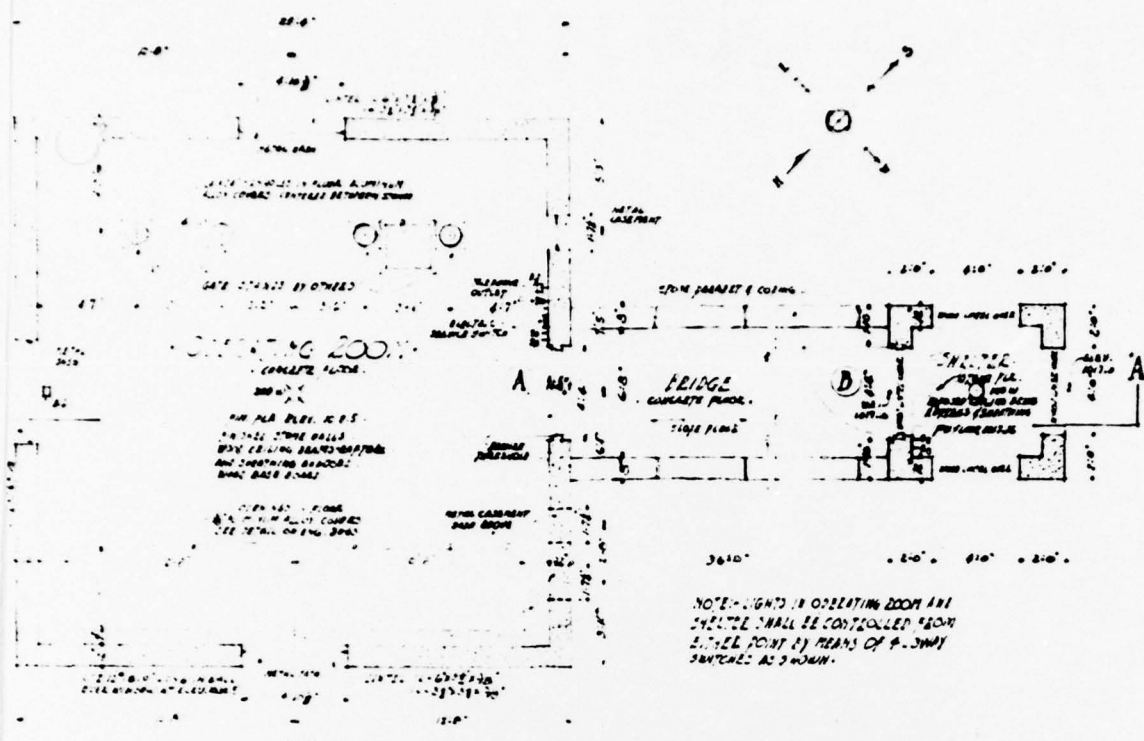
WORKING DRAWING NO 9 G

Scale  $\frac{1}{4}'' = 1'$

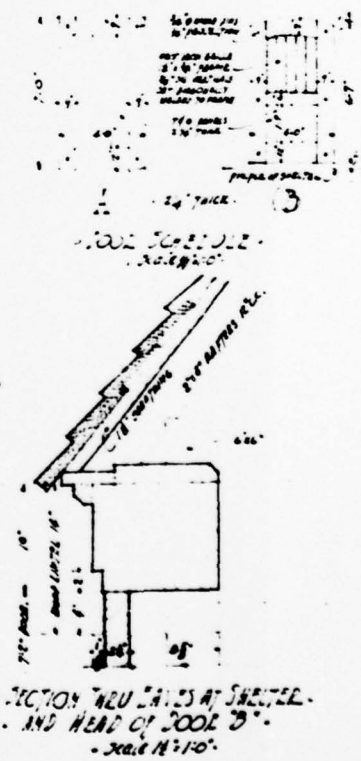
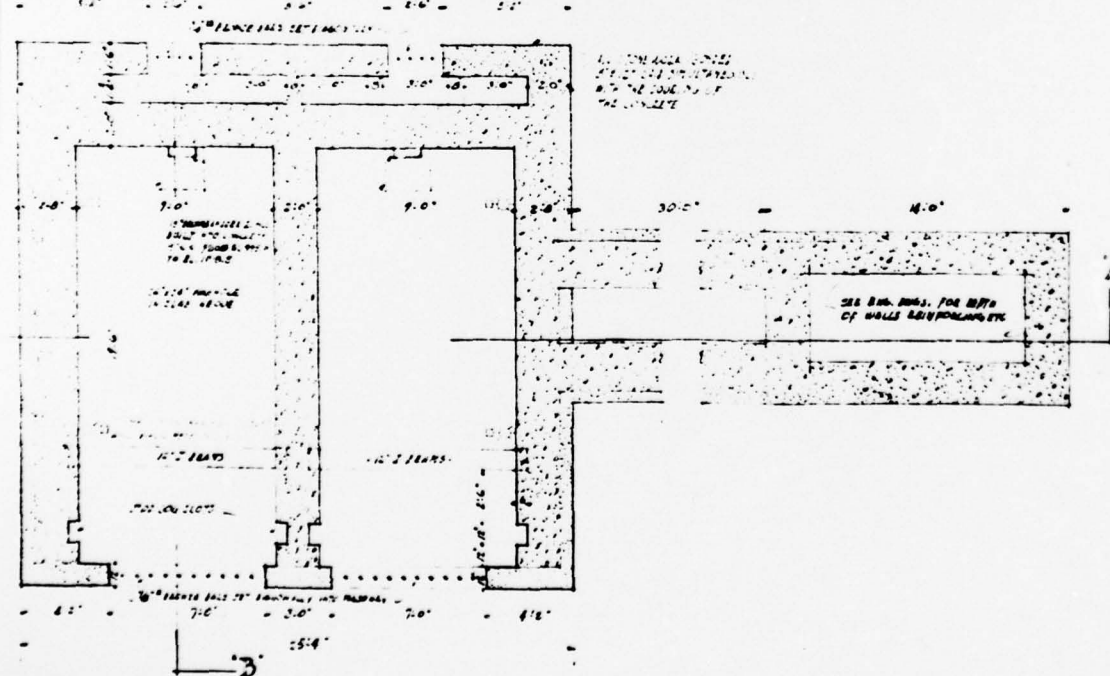
Feb 20, 1932

FIGURE 10

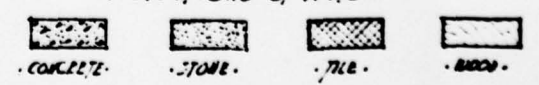




PLAN TWO OPERATING ROOM & SHELTER - SHOWING CONNECTING DOOR - SCALE 1/4" = 1'-0"

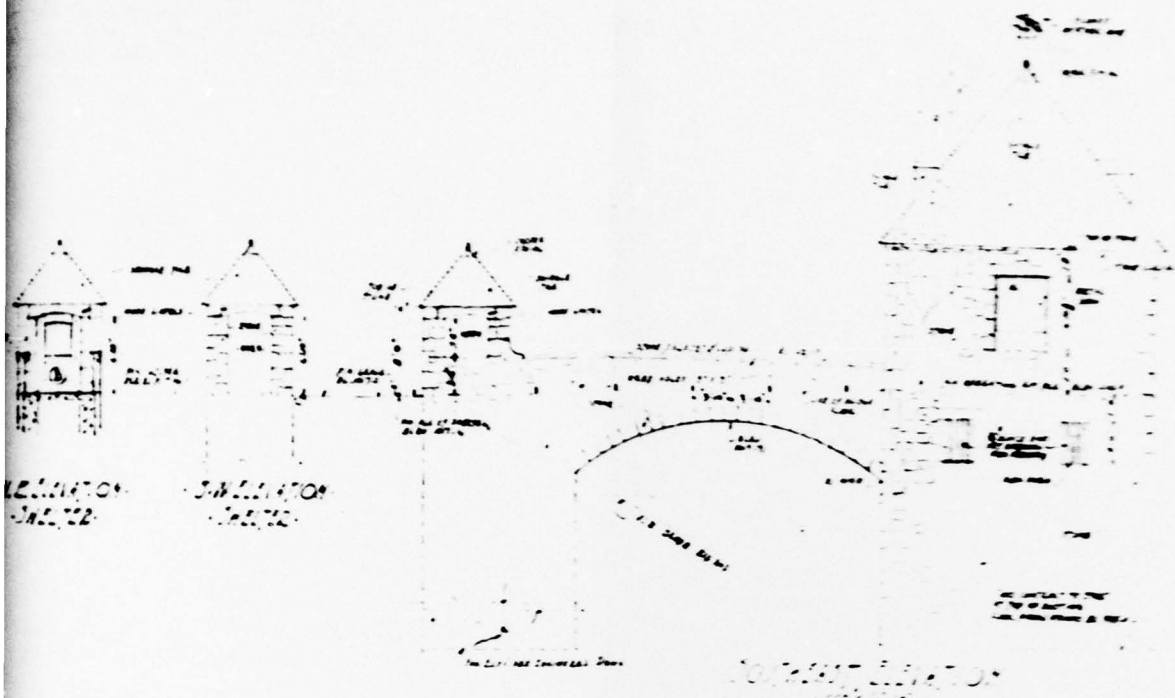


INDICATIONS OF MATERIALS -

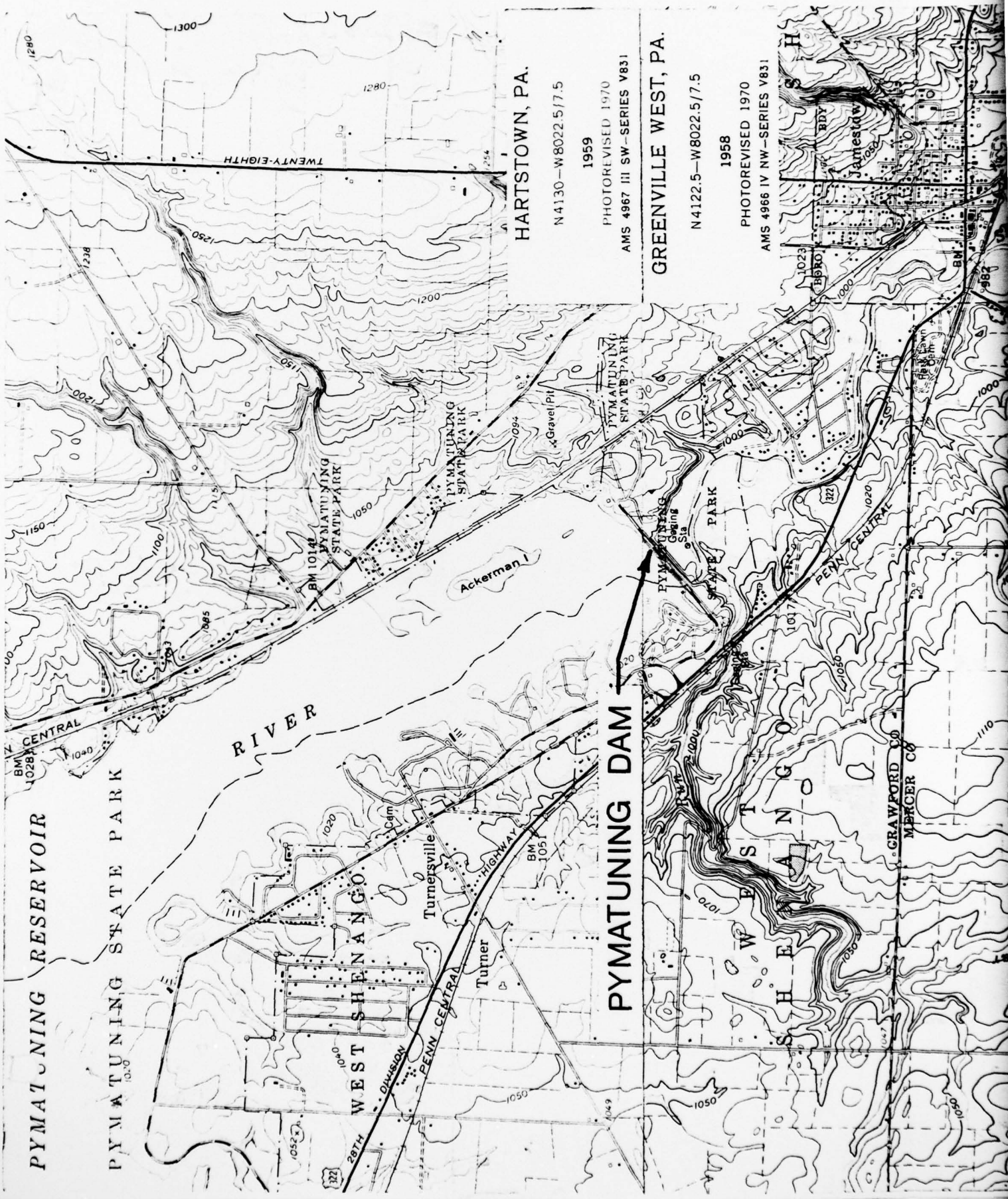


NOTE: SEE ENGINEER'S DRAWINGS FOR OTHER CONSTRUCTION DETAILS - SIZES OF ALL BRICKS, SLABS AND WALLS AND REINFORCING AND STRUCTURAL STEEL BELOW ELEVATION 1010.5'





NOTES: LINE CHANGES  
 1. 1/2" LINE CHANGES  
 2. 1/4" LINE CHANGES  
 3. 1/8" LINE CHANGES  
 4. 1/16" LINE CHANGES  
 5. 1/32" LINE CHANGES  
 6. 1/64" LINE CHANGES  
 7. 1/128" LINE CHANGES  
 8. 1/256" LINE CHANGES  
 9. 1/512" LINE CHANGES  
 10. 1/1024" LINE CHANGES  
 11. 1/2048" LINE CHANGES  
 12. 1/4096" LINE CHANGES  
 13. 1/8192" LINE CHANGES  
 14. 1/16384" LINE CHANGES  
 15. 1/32768" LINE CHANGES  
 16. 1/65536" LINE CHANGES  
 17. 1/131072" LINE CHANGES  
 18. 1/262144" LINE CHANGES  
 19. 1/524288" LINE CHANGES  
 20. 1/1048576" LINE CHANGES  
 21. 1/2097152" LINE CHANGES  
 22. 1/4194304" LINE CHANGES  
 23. 1/8388608" LINE CHANGES  
 24. 1/16777216" LINE CHANGES  
 25. 1/33554432" LINE CHANGES  
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 28. 1/268435456" LINE CHANGES  
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WEST VIRGINIA

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